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Maple Bridge Subdivision, Phase 2

PRELIMINARY STORMWATER MANAGEMENT REPORT

Mason Homes Limited

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Document Contents

1	Introduction	1
1.1	Report Objective	1
1.2	Guidelines & Background Reports	1
2	Development Site.....	3
2.1	Location	3
2.2	Site Description	3
2.3	Proposed Development.....	4
3	Existing Drainage Conditions	6
3.1	Background Information.....	6
3.2	Drainage Patterns	6
3.3	Hydrology	6
4	Stormwater Management Design Criteria	8
5	Stormwater Management Plan	10
5.1	Drainage Conditions	10
5.2	Quantity Control	11
5.3	Conveyance	13
5.4	Quality Control.....	13
5.5	Runoff Volume Control.....	14
5.6	Water Balance	15
5.7	Phosphorus Treatment & Mitigation	16
5.8	Erosion & Sediment Control	17
6	Summary.....	19



Tables

Table 1: Existing Conditions Peak Flow Summary 7

Table 2: SWMF #1 Operating Characteristics 12

Table 3: Post-Development Conditions Peak Flow Summary 13

Table 4: Phosphorus Loading Summary 17

Figures

Figure 1: Site Location Plan 20

Appendices

- Appendix A: Draft Plan
- Appendix B: Existing Conditions Analysis
- Appendix C: Proposed Condition Analysis
- Appendix D: Stage-Storage-Discharge Tables
- Appendix E: Water Quality Calculations
- Appendix F: Soakaway Pit Design
- Appendix G: Phosphorus Balance

Drawings

- Drawing CDP-1: Concept Development Plan
- Drawing DP-1: Pre-Development Drainage Plan
- Drawing DP-2: Post-Development Drainage Plan
- Drawing PND-1: Stormwater Management Facility



1 Introduction

Tatham Engineering Limited (Tatham) was retained by Mason Homes Limited to prepare a Preliminary Stormwater Management (SWM) Report in support of a Draft Plan of Subdivision application for Phase 2 of the Maple Bridge Subdivision located northeast of the Centre Road and Oakeside Drive intersection in the Township of Uxbridge (Township), within the Regional Municipality of Durham (Region).

1.1 REPORT OBJECTIVE

This report was prepared to demonstrate the proposed development will not adversely affect local stormwater quantity or stormwater quality conditions. This will be accomplished by evaluating the effect of the proposed development on local drainage conditions and, where necessary, providing solutions to mitigate any adverse impacts. The feasibility of providing appropriate SWM controls will be demonstrated.

1.2 GUIDELINES & BACKGROUND REPORTS

This report is prepared in consideration of the following guidelines and documents:

- The Ministry of the Environment, Conservation, and Parks (MECP, formerly known as Ministry of Environment), *Stormwater Management Practices Planning and Design Manual* (2003);
- The Ministry of the Environment, Conservation, and Parks (MECP, formerly known as Ministry of Environment), *Lake Simcoe Protection Plan* (LSPP) (2009);
- Lake Simcoe Region Conservation Authority (LSRCA), *Technical Guidelines for Stormwater Submissions* (2022);
- Lake Simcoe Region Conservation Authority (LSRCA), *Phosphorus Offsetting Policy* (2025);
- The Township of Uxbridge, *Zoning By-Law Number 81-19 (As Amended) of the Corporation of the Township of Uxbridge*, Township of Uxbridge (2019);
- The Township of Uxbridge, *Stormwater Management Master Plan - Uxbridge Urban Area and Hamlet of Coppin's Corners* (2016); and
- Regional Municipality of Durham, *Design and Construction Specifications for Regional Services* (2023).

This report is prepared in consideration of the following site-specific reports:

- GHD, *Hydrogeological Assessment Addendum* (February 2025);



- Tatham Engineering Limited, *Maple Bridge Subdivision, Phase 2: Functional Servicing Report* (August 2025); and
- Tatham Engineering Limited, *Maple Bridge Subdivision, Phase 2: Natural Hazards Assessment* (October 2024).



2 Development Site

2.1 LOCATION

As illustrated in Figure 1, the subject site is located at the property known municipally as 7309 Centre Road, Township of Uxbridge.

As per the boundary survey completed by H.F. Grander Co. Ltd. in October 2022, the site is legally described as:

(Parts 1, 2, 3, Plan 40R-21667
Save & Except Parts 1, 2, 3, & 4, Plan 40R-23402,
Save & Except Parts 1, 2, 3, 4, 5, 6, & 7, Plan 40R-23403,
And Save & Except Plan 40M-2256)
Of Part of Lot 33, Concession 6,
Geographic Township of Uxbridge,
Now in the, Township of Uxbridge,
Regional Municipality of Durham

2.2 SITE DESCRIPTION

2.2.1 Zoning & Land Uses

Based on the Township's current Official Plan the property is designated 'Future Residential Area' and 'Natural Hazard Area'. Based on the Township's Zoning By-law the property is within a Rural Zone (RU) and Environmental Protection Zone (EP-27).

The site is located within the LSRCA watershed and is partially located within the LSRCA regulated area due to natural hazards associated with the Uxbridge Brook.

2.2.2 Topography

Information relating to existing topography, ground cover, and drainage patterns was obtained through a review of relevant background studies, available plans, base mapping, and topographic surveys. A detailed topographic survey of the site was completed by IBW Surveyors in September 2019. This survey has been reviewed and compared to other available contour mapping and appears to be sufficient for preliminary design. If required, additional topographic survey will be completed during the detailed design stage.

The subject property is approximately 13.58 ha in area and consists mainly of agricultural croplands with areas of woodland. It is bound by existing agricultural lands to the north and southwest, Tributary 6.1 of the Uxbridge Brook to the south and east, Tributary 4.0 of the Uxbridge Brook to the north, and Centre Road to the west.



Under existing conditions, runoff from the Phase 2 lands generally flow overland in an east and southeast direction towards Tributary 6.1 of the Uxbridge Brook, which meanders between the subject site and neighbouring Phase 1 lands immediately south of the watercourse.

2.2.3 Geotechnical & Hydrogeological Setting

The *Hydrogeological Assessment Addendum* identified the subsurface conditions as topsoil underlain by a thin layer of silty sand/sandy silt and native glacial till. Groundwater monitoring well readings were recorded in four monitoring wells at depths ranging from 0.57 to 3.65 mbgs, indicating groundwater generally flows southeast across the site.

Based on infiltration testing results conducted by GHD, in-situ infiltration rates range from 39 to 56 mm/hr. Design infiltration rates were established to range from 16 to 23 m/hr which was utilized in the low impact development design (LID) calculations described in Section 5.5 and 5.6 of this report.

Due to high groundwater levels, LID measures will be implemented in select areas of the proposed development only. Refer to the Conceptual Soakaway Pit Locations drawing in Appendix F for reference. Opportunities to increase LID measures on-site will be assessed during detailed design.

2.3 PROPOSED DEVELOPMENT

2.3.1 Land Use

The proposed residential development consists of the following:

- 82 townhouse units;
- 154 single family dwellings; and
- a 0.79 ha stormwater management block.

The Draft Plan also includes Open Space blocks, Walkway blocks, an Environmental Protection block, and a Future Road Connection block.

The proposed development surrounds an existing 0.9 ha residential property fronting Centre Road (owned by others). Mason Homes Limited intends to purchase the property in the future with the intention of developing it with a similar built form to what is proposed for the subject site. The proposed Draft Plan of Subdivision does not include development of this property. However, the proposed preliminary SWM plan for the subject site accounts for the development of this 0.9 ha parcel.

Refer to Appendix A for the proposed Draft Plan.



2.3.2 Access

The site will be accessed from two new connections to existing roadways. The west connection will be made from Street 'A,' which is a 20 m right-of-way, in the northwest corner of the development to Centre Road. The east connection will be made from Street 'A' in the northeast corner of the development to Oakside Drive.

2.3.3 Internal Roads

The internal roadways will be a combination of 20 m road allowances, 17 m road allowances and 7.5 m laneways.



3 Existing Drainage Conditions

3.1 BACKGROUND INFORMATION

Existing topography, ground cover, soil condition, land use and drainage patterns for the site and the surrounding area were established through a review of available aerial photography, topographic survey and relevant background reports.

3.2 DRAINAGE PATTERNS

The development site consists of approximately 14.5 ha of undeveloped agriculture land with forested areas along the property boundaries. The site generally drains east and southeast towards Tributary 6.1 of the Uxbridge Brook at a gentle slope (2% to 5%), while a small portion of the lands drains north towards Tributary 4.0 of the Uxbridge Brook at a significantly steeper slope (upwards to 30%+).

Two existing conditions drainage catchments (Catchment 100 and 101) have been delineated using base mapping and detailed topographic survey information and are illustrated on the Pre-Development Drainage Plan (Drawing DP-1) enclosed for reference.

- Catchment 100 is approximately 0.62 ha and consists of woodland, meadows, and cultivated lands. The catchment generally drains north to Tributary 4.0 of Uxbridge Brook, which has been denoted as Outlet #2.
- Catchment 101 is approximately 13.85 ha and consists of woodland, lawns, meadows, cultivated lands, and an existing residential development at 7309 Centre Road. The catchment generally drains south and east to Tributary 6.1 of Uxbridge Brook, which has been denoted as Outlet #1.

3.3 HYDROLOGY

In accordance with the Township and LSRCA requirements, a Visual OTTHYMO (VO6.2) model has been prepared to estimate the pre-development peak flows for the 1:2-year through 1:100-year storm events. Site-specific Intensity-Duration Frequency (IDF) rainfall data was generated using the *IDF Curve Look-up Tool* in accordance with Township standards. The design storms were modeled with a 4-hour CHI, 12-hour SCS, and 24-hour SCS distribution under AMCII conditions. In addition, the Regional storm (Hurricane Hazel) storm has been modelled under AMCI conditions, in accordance with LSRCA requirements.

The existing condition peak flows from the site at Outlet #1 and Outlet #2 are summarized in Table 1 while detailed model results are included in Appendix B for reference. Detailed



calculations of the existing condition hydrological modelling parameters are also included in Appendix B for reference.

Table 1: Existing Conditions Peak Flow Summary

DESIGN STORM	OUTLET #1 (m ³ /s)			OUTLET #2 (m ³ /s)		
	4-hr CHI	12-hr SCS	24-hr SCS	4-hr CHI	12-hr SCS	24-hr SCS
25 mm		0.03			0.00	
1:2-year	0.06	0.13	0.17	0.01	0.02	0.02
1:5-year	0.11	0.24	0.30	0.01	0.03	0.04
1:10-year	0.16	0.32	0.40	0.02	0.04	0.05
1:25-year	0.23	0.44	0.54	0.03	0.05	0.07
1:50-year	0.28	0.53	0.66	0.04	0.07	0.09
1:100-year	0.34	0.63	0.78	0.04	0.08	0.10
Hurricane Hazel		1.48			0.08	

Hurricane Hazel peak flows presented in this table are representative of AMCIII conditions.



4 Stormwater Management Design Criteria

The SWM plan is subject to review and approval from the Township and the LSRCA. The SWM plan has been developed in accordance with the following Township and LSRCA design criteria:

Quantity Control

Post-development peak flow rates must be controlled to pre-development rates or less for storm events up to and including the 1:100-year event to ensure no adverse impacts for downstream landowners at all site outlets.

Conveyance

Under proposed conditions, the 1:100-year return frequency design storm from subject property must be safely conveyed to the site outlet. In addition, the Regional Storm peak flows for the upstream development must be safely conveyed through the site to the site outlet.

Quality Control

Water quality controls must be provided to satisfy the *MECP Stormwater Management Planning and Design Manual*. Enhanced level water quality protection, which corresponds to 80% long term suspended solids (TSS) removal, is required.

Runoff Volume Control

In accordance with LSRCA policies, any works meeting the definition of 'major development' are required to meet the volume control requirements as outlined in Section 3.2.4 of the *Technical Guidelines for Stormwater Management Submissions*. Best efforts must be demonstrated to infiltrate, filter or re-use the 25 mm storm event runoff from impervious areas on site.

Water Balance

As the proposed development is categorized as a 'major development' under the LSPP, best efforts must be demonstrated towards maintaining pre-development infiltration rates under post-development conditions.

Phosphorus Treatment & Mitigation

The proposed development is categorized as a 'major development' and in accordance with LSRCA requirements, the site will be subject to *the Lake Simcoe Phosphorus Offsetting Policy* which requires all 'major' development projects to control post-development phosphorus loading rates to pre-development rates. Any remaining phosphorus loadings that cannot be mitigated will be subject to a cash-contribution for off-site mitigation.

Erosion & Sediment Control

An erosion and sediment control plan is required for the subject development to demonstrate erosion mitigation measures to manage the risk of sediment transport downstream. In addition,



the LSRCA requires runoff generated from the 25 mm storm be detained and released over a period of at minimum 24 hours as described in Section 3.4 of the *Technical Guidelines for Stormwater Management Submissions*.



5 Stormwater Management Plan

Under proposed conditions, the 14.5 ha site will generally be graded to convey runoff to the proposed SWM pond at the east end of the development.

5.1 DRAINAGE CONDITIONS

The subject property has been divided into four drainage catchments under proposed conditions (Catchments 201, 202, and 203) as illustrated on the Post-Development Drainage Plan (Drawing DP-2) enclosed for reference.

- Catchment 201 is approximately 12.44 ha and includes a majority of the proposed development. Minor flows from this catchment will be conveyed to the SWM pond via storm sewer while major flows will be conveyed to the pond overland via the road right-of-way. The design of the internal storm sewer will occur at detailed design. Attenuated peak flows are discharged to Outlet #1 located south of the site, which is a sufficient outlet. Catchment 201 was delineated to maximize peak flows contributing to the SWM pond by accounting for rooftop and driveway drainage from the proposed lots located within Catchment 202. An assumed imperviousness of 100% was used for the full buildout of the 0.91 ha residential block to accommodate for future development.
- Catchment 202 is approximately 1.60 ha and consists of rear yards, landscape, and forested areas. This catchment will drain uncontrolled to Outlet #1 (Uxbridge Brook Tributary 6.1). Due to grading constraints, Catchment 202 was delineated such that rooftop drainage from lots located within this catchment will be conveyed to the SWM pond.
- Catchment 203 is approximately 0.43 ha and consists of a small portion of Street 'A' and an open space block. Due to grading constraints this catchment will drain uncontrolled to Outlet #1 (Uxbridge Brook Tributary 6.1).

It is noted that per LSRCA *Technical Guidelines for Stormwater Submission*, peak flow rates generated on site must be conveyed to a sufficient outlet. Due to site constraints, Outlet #2 (Uxbridge Brook Tributary 4.0) does not meet the requirements for a sufficient outlet since flows would drain through private property to the north. Therefore, the site been graded to drain all generated peak flows to Outlet #1 (Uxbridge Brook Tributary 6.1). Since peak flows from Catchments 201, 202, and 203 are discharged directly to Outlet #1, this requirement is met.

The hydrologic modelling parameters for proposed conditions have been calculated based on the proposed site plan, standard ROW cross-sections, and the maximum lot coverages defined in the *Zoning By-Law Number 81-19 (As Amended) of the Corporation of the Township of*



Uxbridge and engineering judgement. The preliminary proposed condition hydrologic parameter calculations are included in Appendix C for reference.

5.2 QUANTITY CONTROL

As the proposed development will result in an increase in impervious areas (roads, buildings and driveways), on-site stormwater quantity controls are required to reduce post-development peak flow rates to the allowable release rates or less.

Water quantity controls will be provided by an end-of-pipe SWM pond to attenuate proposed peak flows to pre-development levels. Due to grading constraints, Catchments 202 and 203 will drain uncontrolled to Outlet #1. As such the SWM pond has been sized to over-control the peak flows from Catchment 201 such that the post-development peak flows at the site outlets are below pre-development peak flows, accounting for the uncontrolled flow.

Peak flow attenuation will be provided by the proposed SWM wet pond which consists of a 120 mm orifice plate with an elevation of 267.00 m, a ditch inlet catch basin (DICB) with top of grate at elevation of 267.80 with a secondary 370 mm dia. orifice plate with invert of 267.00 m, an outlet pipe with invert of 266.85 m, and an emergency overflow weir set at 268.90 m. During the 1:100-year design storm, the pond will provide an active storage volume of 6,442 m³ at an elevation of 268.52 m with 0.78 m of freeboard. During Regional Storm Hurricane Hazel (AMCIII conditions), the pond will provide 8,809 m³ of storage at elevation 269.00 m with 0.30 m freeboard.

A summary of additional preliminary design details is provided below:

- Bottom of Wet Cell: 265.00 m
- Bottom of Forebay: 266.00 m
- Permanent Pool Elevation: 267.00 m
- Top of Pond Elevation: 269.30 m
- Total Storage at Top of Pond Elevation: 10,646 m³

The emergency overflow spillway has been designed to provide safe conveyance of the uncontrolled 1:100-year storm event (Regulatory storm) in the event the outlet structures are fully blocked. The weir will safely convey the flows at an elevation of 269.10 m.

Additional outlet design details will be provided at the detailed design stage.



Table 2: SWMF #1 Operating Characteristics

DESIGN STORM	STORAGE VOLUME (m ³)			DISCHARGE (m ³ /s)			WATER ELEVATION (m)		
	4-hr	12-hr	24-hr	4-hr	12-hr	24-hr	4-hr	12-hr	24-hr
	CHI	SCS	SCS	CHI	SCS	SCS	CHI	SCS	SCS
25 mm		1,241			0.02			267.36	
1:2-year	1,846	2,584	2,860	0.02	0.03	0.04	267.51	267.69	267.75
1:5-year	2,712	3,405	3,750	0.04	0.08	0.13	267.72	267.88	267.95
1:10-year	3,177	3,839	4,228	0.06	0.15	0.26	267.82	267.97	268.06
1:25-year	3,681	4,380	4,977	0.12	0.31	0.34	267.94	268.09	268.22
1:50-year	3,913	4,893	5,704	0.16	0.34	0.36	267.99	268.20	268.37
1:100-year	4,134	5,521	6,442	0.23	0.36	0.38	268.04	268.33	268.52
Hurricane Hazel		8,809			1.72			269.00	

Hurricane Hazel peak flows presented in this table are representative of AMCIII conditions.

The preliminary stage-storage-discharge tables for the SWM pond are included in Appendix D for reference. Refer to the Stormwater Management Facility Plan (Drawing PND-1), appended, for preliminary design details. It is noted that the outlet elevation of the pond (266.85 m) has been set above the Regional flood elevation (266.60 m), as shown on Drawing PND-1.

Due to the implementation of on-site water quantity controls, post-development peak flow rates at Outlet #1 and Outlet #2 are less than or equal to the existing peak flows leaving the site for the selected design storms. A summary of post-development peak flows at Outlet #1 and Outlet #2 is provided in Table 3 while detailed model results are included in Appendix C for reference.



Table 3: Post-Development Conditions Peak Flow Summary

DESIGN STORM	OUTLET #1 (m ³ /s)			OUTLET #2 (m ³ /s)		
	4-hr CHI	12-hr SCS	24-hr SCS	4-hr CHI	12-hr SCS	24-hr SCS
25 mm		0.02 (0.03)			0.00 (0.00)	
1:2-year	0.04 (0.06)	0.07 (0.13)	0.09 (0.17)	0.00 (0.01)	0.00 (0.02)	0.00 (0.02)
1:5-year	0.07 (0.11)	0.11 (0.24)	0.14 (0.30)	0.00 (0.01)	0.00 (0.03)	0.00 (0.04)
1:10-year	0.09 (0.16)	0.16 (0.32)	0.28 (0.40)	0.00 (0.02)	0.00 (0.04)	0.00 (0.05)
1:25-year	0.13 (0.23)	0.34 (0.44)	0.41 (0.54)	0.00 (0.03)	0.00 (0.05)	0.00 (0.07)
1:50-year	0.18 (0.28)	0.40 (0.53)	0.51 (0.66)	0.00 (0.04)	0.00 (0.07)	0.00 (0.09)
1:100-year	0.25 (0.34)	0.49 (0.63)	0.61 (0.78)	0.00 (0.04)	0.00 (0.08)	0.00 (0.10)
Hurricane Hazel		1.97 (1.48)			0.00 (0.08)	

Hurricane Hazel peak flows presented in this table are representative of AMCIII conditions.
 Values in *italics* represent existing peak flow rates.

5.3 CONVEYANCE

As previously mentioned, major and minor peak flows generated from the upstream development will be collected internally and conveyed to the SWM pond. The Regulatory storm peak flows must be safely conveyed through the site to the site outlet. In addition, the design of the internal storm sewer will occur at detailed design.

Under an emergency scenario in which the storm sewers are 100% blocked, a conveyance channel located in the 6.0 m servicing easement (between lots 51 and 52) is proposed to provide safe conveyance of flows to the wet pond. The Regulatory storm (the uncontrolled 1:100-year 24-hour SCS storm event of 3.6 m³/s) can be sufficiently conveyed through the conveyance channel. Supporting calculations are provided in Appendix C.

5.4 QUALITY CONTROL

“Enhanced” level water quality control corresponding to 80% TSS removal is required for developed areas of the site. Water quality controls for the majority of the development will be provided by the SWM pond which has been designed as a wet pond with a sediment forebay to provide adequate water quality storage volumes based on the MOE *Stormwater Management Practices Planning and Design Manual* (2003) Table 3.2. The wet pond has been designed to



provide “Enhanced” Level 1 water quality treatment for all contributing drainage from the upstream catchments.

The SWM pond was designed using a drainage area of 12.4 ha and an imperviousness of 71% contributing from the subject development. Therefore, based on Table 3.2 of the MOE *Stormwater Design Manual*, the required water quality storage volume is 176 m³/ha (or 2,194 m³ for the 12.4 ha area). Of this volume, 1,696 m³ is required for the permanent pool and 498 m³ (or 40 m³/ha for the 12.4 ha area) is required for extended detention. The provided permanent pool and extended detention volumes are 3,940 m³ and 3,068 m³ respectively. As such the pond is adequately sized to provide the required “Enhanced” level water quality control. Detailed water quality calculations are included in Appendix E for reference. Based on the available groundwater information, it is noted the SWM pond may need to be lined to avoid groundwater interaction with the permanent pool. This will be further assessed during detailed design.

In addition, a drawdown time of 40 hours and 69 hours is provided for the 25 mm storm event and the extended detention zone respectively, which satisfies LSRCA’s Stream Erosion Control requirements in Section 3.4 of the *Technical Guidelines for Stormwater Management Submissions*.

Catchment 202 consists of only rear yards and forested areas and as such, water quality treatment is not required. Due to grading constraints, runoff from Catchment 203 is unable to be conveyed to the SWM pond and will be released uncontrolled to Outlet #1. The size of this catchment has been minimized through the site grading to reduce the untreated area. This catchment will be further assessed at detailed design to further minimize the uncontrolled area, if possible.

5.5 RUNOFF VOLUME CONTROL

In accordance with LSRCA requirements, projects defined as ‘major developments’ are required to meet the volume control requirements as outlined in Section 3.2.4 of the *Technical Guidelines for Stormwater Management Submissions*. As such, best efforts must be provided to infiltrate, filter, or re-use runoff generated from impervious areas on site.

Due to constraints posed by the latest available high groundwater data, it is anticipated that infiltration of the full 25 mm storm event over impervious areas is unfeasible. Refer to the Conceptual Soakaway Pit Locations drawing in Appendix F for reference, for a depiction of feasible infiltration practice locations on site.

As such, the retention of the 25 mm storm for rooftops has been proposed where feasible based on groundwater levels. Where feasible, lot-level soakaway pits are proposed to capture the 25 mm runoff from the rooftops of the single-family dwelling units and 50% of the 25 mm rooftop



runoff from the townhouse units. This corresponds to 0.77 ha of impervious drainage area to the soakaway pits. The proposed soakaway pits will provide a total storage volume of 195 m³. As the total impervious area of the site is 8.2 ha, the soakaway pits will provide runoff control equivalent to the 2.4 mm storm across the site. This design satisfies “Alternative #3” criteria as specified in Section 3.2.6 of the *Technical Guidelines for Stormwater Management Submissions*.

Per recommendations of the *Hydrogeological Assessment Addendum*, the infiltration rate used for the design of the soakaway pits is 16 mm/hr which equates to a drawdown time of 30 hours.

Preliminary soakaway pit sizing and drawdown time calculations are included in Appendix F for reference. Preliminary cross-sectional details are provided in the Conceptual Soakaway Pit Locations drawing enclosed in Appendix F of this report.

It is noted that the 0.9 ha residential block was excluded from this runoff volume controls assessment since infiltration is provided on an individual lot-level basis. Should this block be assumed by Mason Homes Limited in the future, the feasibility of runoff volume controls for this block will be assessed.

Additional opportunities to increase infiltration/filtration will be explored in support of the detailed design such as additional soakaway pits or other infiltration features where feasible. Design details including locations and sizing will be confirmed accordingly.

5.6 WATER BALANCE

In accordance with the LSPP, an evaluation of the anticipated changes in the water balance between pre-development and post-development conditions has been included to demonstrate how the proposed SWM control measures will minimize changes to water balance across the site.

GHD Group Pty Ltd (GHD) has prepared a water balance as part of their *Hydrogeological Assessment Addendum*. As part of the assessment, annual runoff and infiltration volumes were calculated under pre- and post-development conditions, and post-development conditions with mitigation measures. Per their assessment, the pre-development infiltration volume is 20,187 m³/year and the post-development infiltration volume is 6,171 m³/year which represents an approximately 70% decrease.

The proposed soakaway pits can capture a maximum volume of 195 m³, equivalent to the 25 mm storm event captured on rooftops where infiltration is feasible. This is equivalent to an annual volume capture after evapotranspiration of 5,088 m³/year which brings the post-development infiltration volume with mitigation measures to 11,259 m³/year. This represents a 44% reduction in annual infiltration, representing best efforts to increase post-development infiltration and mitigate the annual infiltration deficit as much as feasibly possible.

Refer to the GHD's report under separate cover for additional details.



5.7 PHOSPHORUS TREATMENT & MITIGATION

An assessment of the total phosphorus (TP) loading from the site under existing and proposed conditions has been completed using the Low Impact Development Treatment Train Tool (LID-TTT).

It is noted that the 0.9 ha residential block was excluded from this phosphorus loading assessment since controls will be provided on an individual lot-level basis in addition to the SWM pond. Should this block be assumed by Mason Homes Limited in the future, the phosphorus loading for this block will be assessed.

Existing Conditions

Under existing conditions, the site has been modelled as paved surface (TP concentration of 0.23 mg/L), landscaped area (TP concentration of 0.32 mg/L), row crop (TP concentration of 0.23 mg/L), open space (TP concentration of 0.20 mg/L), and forest (TP concentration of 0.23 mg/L). The existing TP loading rates at Outlet #1 and Outlet #2 are 2.14 kg/year and 0.10 kg/year respectively, resulting in a total existing TP loading rate of 2.25 kg/year.

Proposed Conditions

Under proposed conditions, the site has been modelled as paved surface, rooftop (TP concentration of 0.09 mg/L), landscaped area, forest, and wet pond (TP concentration of 0.81 mg/L). The proposed TP loading rate without mitigation at Outlet #1 is 15.74 kg/year. Due to site grading, the TP loading rate at Outlet #2 is reduced to 0 kg/year. Therefore, the total proposed TP loading rate on a site-basis is 15.74 kg/year.

Proposed Conditions with Mitigation

TP mitigation for the site will be provided via lot-level soakaway pits and the wet pond which have median TP removal efficiency rates of 87% and 63% respectively.

Select rooftop areas corresponding to 0.77 ha from Catchment 201 will drain to the lot-level soakaway pits while the remaining 10.76 ha (excluding the 0.9 ha residential block) will drain directly to the wet pond. Due to grading constraints Catchments 202 and 203 are released uncontrolled to Outlet #1.

Therefore, the estimated total annual post-development TP loading rate is reduced to 5.59 kg/year which represents a 64% net reduction. The remaining 36% TP removal can be achieved through additional soakaway pits, enhanced grass swales, and/or filtration treatment device(s) such as a Jellyfish Filter unit or approved equivalent, if feasible. Additional treatment methods will be explored at detailed design. Per the *Lake Simcoe Phosphorus Offsetting Policy*,



any remaining TP loadings that cannot be mitigated will be subject to a cash-contribution for off-site mitigation.

A summary of the TP loadings under existing, proposed, and proposed with mitigation scenarios is provided in Table 4 below. The detailed LID-TTT results are provided in Appendix G for reference.

Table 4: Phosphorus Loading Summary

SCENARIO		AREA (ha)	PHOSPHORUS LOADING (kg/year)
Outlet #1	Pre-Development	12.94	2.15
	Post-Development (Without Controls)	13.56	15.74
	Post-Development (With Controls)	13.56	5.59
Outlet #2	Pre-Development	0.62	0.10
	Post-Development (With/Without Controls)	0	0
Total Pre-Development		13.56	2.25
Total Post-Development (Without Controls)		13.56	15.74
Total Post-Development (With Controls)		13.56	5.59

Note: Existing and proposed conditions TP loading areas exclude the 0.91 ha residential block.

5.8 EROSION & SEDIMENT CONTROL

Erosion and sediment control will be implemented for all construction activities within the subject site including vegetation clearing, topsoil stripping, stockpiling of materials, site access construction, grading and servicing. The basic principles considered to minimize erosion and sedimentation, and the potential negative environmental impacts include:

- minimize disturbance activities where possible;
- expose the smallest possible land area to erosion for the shortest amount of time;
- institute erosion control measures as required immediately;
- implement sediment control measures before the outset of construction activities; and
- carry out regular inspection of erosion/sediment control measures and repair or maintain them, as necessary.



Erosion and sediment control measures shall be implemented in accordance with the *Erosion & Sediment Control Best Management Practices Guide* and are to include the following:

- sediment control fence;
- construction access mat;
- heavy-duty silt fence surrounding stripping and material stock pile areas;
- catch basin filter screens; and
- sediment traps placed in all existing and proposed catch basins adjacent to the site.

Regular inspection of control measures will be completed through a monitoring and mitigation plan, with regular repairs made as necessary. An erosion and sediment control plan will be developed during the detailed design stage.

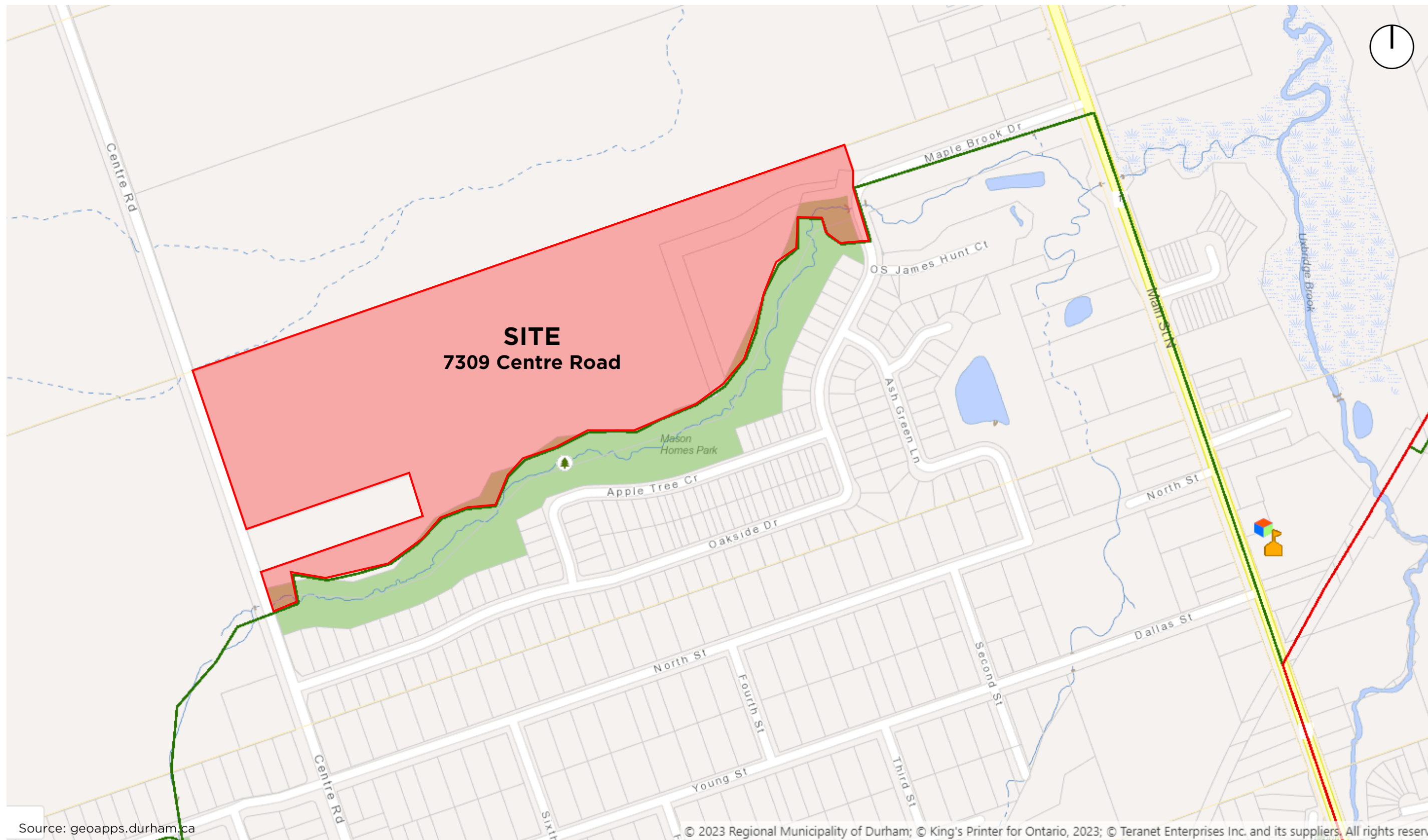


6 Summary

This report has been prepared to document the preliminary drainage and SWM plan for the proposed development. The SWM plan ensures the development can be constructed in accordance with all applicable municipal and provincial guidelines while minimizing the impact of the development on local surface water conditions. The SWM design criteria described in Section 5 of this report will be achieved as detailed below.

- Post-development peak flow rates will be controlled to pre-development rates or less for all storm events at Outlet #1 and Outlet #2. Water quantity storage will be provided via wet SWM pond with sufficient storage to attenuate the proposed peak flows to below pre-development levels.
- “Enhanced” Level 1 water quality controls corresponding to 80% TSS removal will be provided for the proposed development via the wet SWM pond.
- Best efforts have been provided to mitigate the water balance deficit due to the proposed development. The lot-level soakaway pits will provide 25 mm of storage for the treated areas, resulting in an equivalent of 2.4 mm of runoff volume control across the total impervious area of the site. Additional infiltration measures will be explored and implemented where feasible during detailed design.
- Best efforts have also been provided to mitigate phosphorus loadings on site. The proposed wet SWM pond and lot-level soakaway pits will provide approximately 64% reduction in annual phosphorus loading for the site compared to proposed conditions without any mitigation measures. Per the *Lake Simcoe Phosphorus Offsetting Policy*, any remaining phosphorus loadings that cannot be mitigated will be subject to a cash-contribution for off-site mitigation. Additional treatment options will be explored at detailed design.
- A series of erosion and sediment controls including heavy duty silt fence and construction access mats will be implemented for all construction activities.



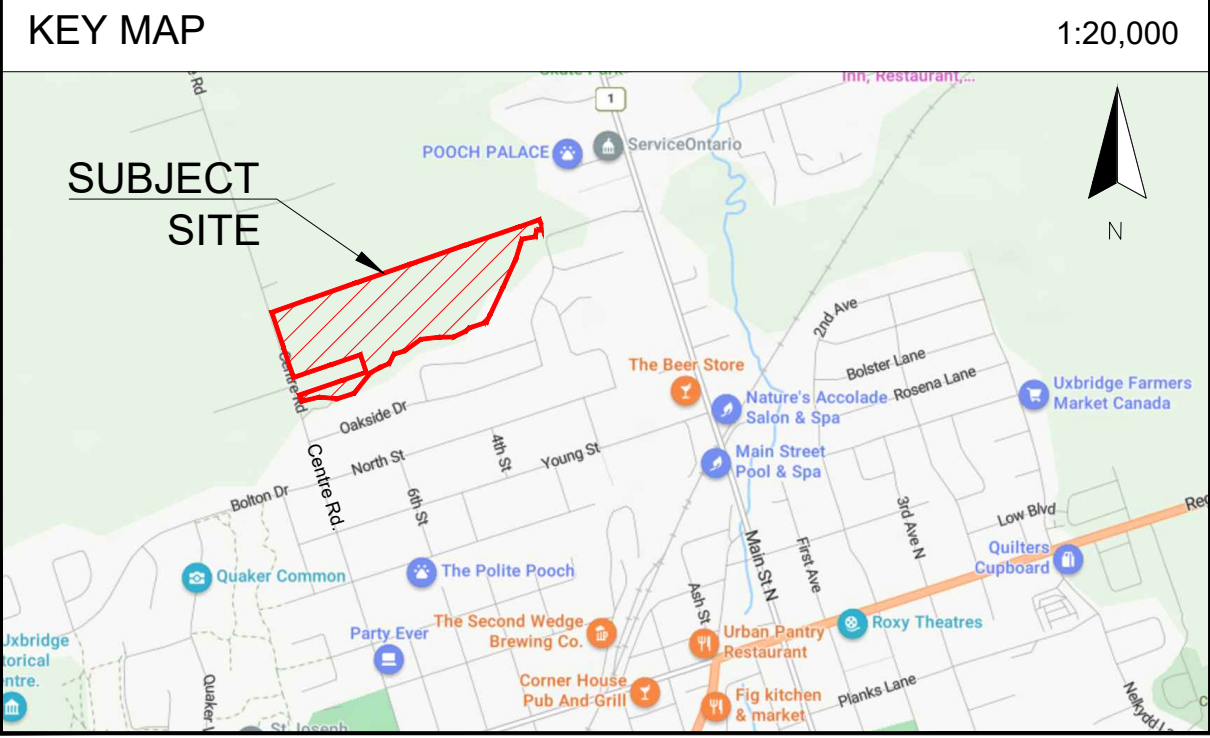
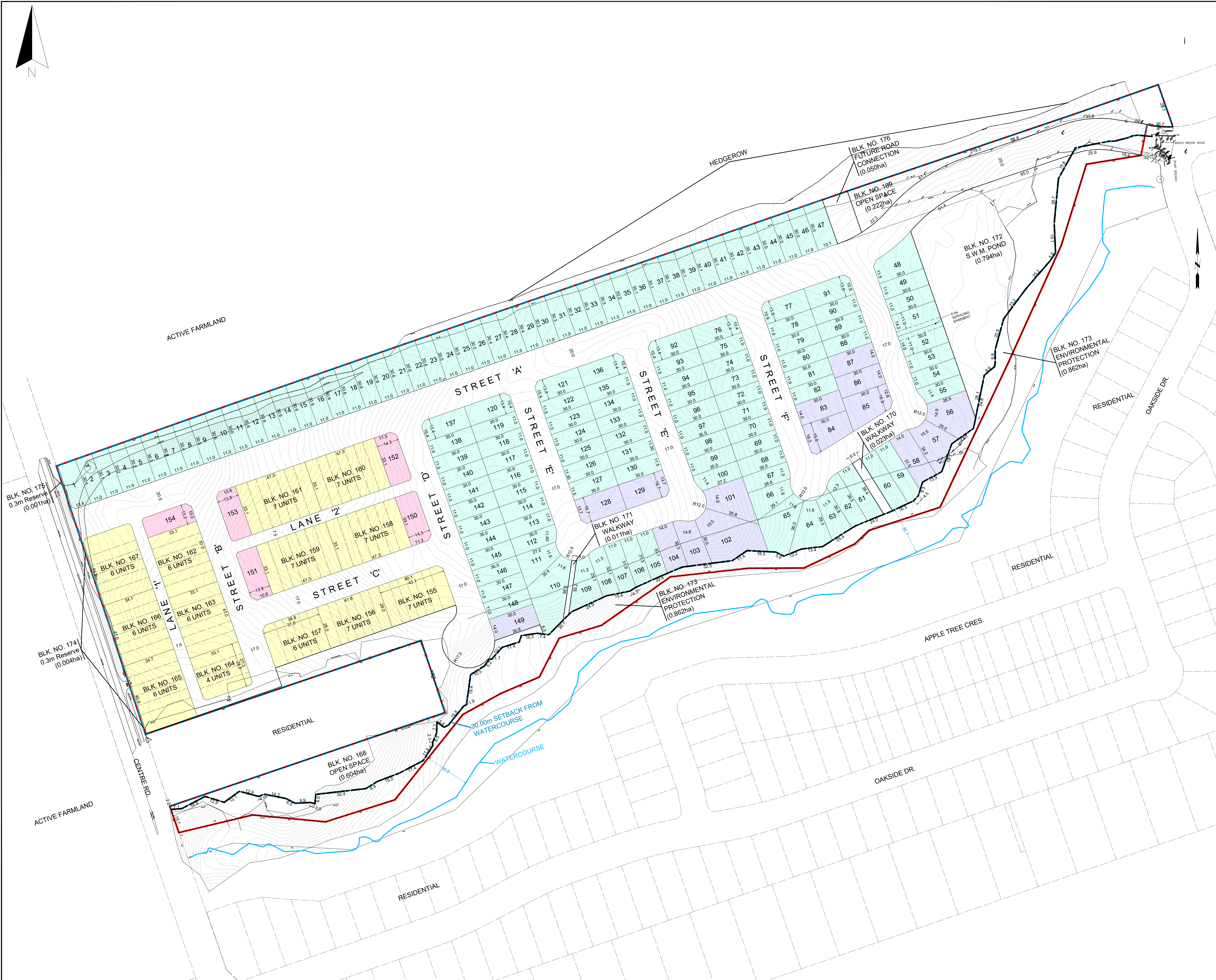


MAPLE BRIDGE PHASE 2 - PRELIMINARY STORMWATER MANAGEMENT REPORT

Figure 1: Site Location

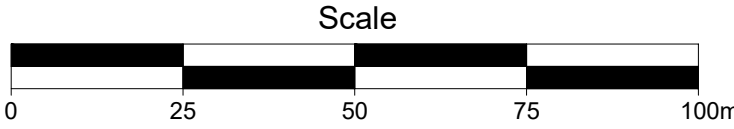


Appendix A: Draft Plan



DRAFT PLAN OF SUBDIVISION

Part of Lot 33, Concession 6,
Township of Uxbridge,
(formerly in the County of Ontario)
Regional Municipality of Durham



LEGEND

SUBJECT LANDS (135,746.80m² / 13.575ha)

OWNER'S CERTIFICATE
I HEREBY AUTHORIZE INNOVATIVE PLANNING SOLUTIONS TO PREPARE THIS DRAFT PLAN OF SUBDIVISION AND SUBMIT THIS DRAFT PLAN OF SUBDIVISION FOR APPROVAL.

DATE 2001976 ONTARIO LIMITED

SURVEYOR'S CERTIFICATE
I CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED AND THEIR RELATIONSHIP TO ADJACENT LANDS ARE ACCURATELY AND CORRECTLY SHOWN.

DATE IVAN B. WALLACE, O.L.S.

ADDITIONAL INFORMATION REQUIRED UNDER SECTION 51(17) OF THE PLANNING ACT

- | | |
|------------------|--|
| a) SHOWN ON PLAN | g) SHOWN ON PLAN |
| b) SHOWN ON PLAN | h) MUNICIPAL WATER |
| c) SEE KEY PLAN | i) SILTY SAND, GLACIAL TILL AND/OR CLAYEY SILT |
| d) RESIDENTIAL | j) SHOWN ON PLAN |
| e) SHOWN ON PLAN | k) MUNICIPAL WATER & SEWAGE |
| f) SHOWN ON PLAN | l) NONE |

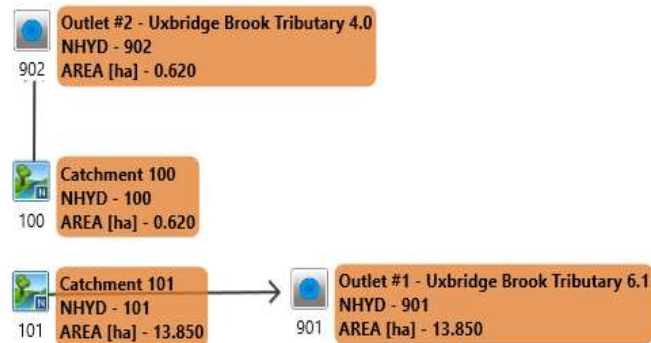
LAND USE STATISTICS			
Land Use	Lot / Blk. No.	Units	Area (ha)
Residential Single Lot (10.97m / 36')	1 - 55, 59 - 82,	134	4.691
	88 - 100, 105 - 127, 130 - 148		
Residential Single Lot (14.02m / 46')	56 - 58, 83 - 87, 101 - 104, 128 - 129, 149	15	0.792
Residential Single Lot - Rear Lane (10.97m / 36')	150 - 154	5	0.226
Residential Townhouses (6.30m / 20.76')	155 - 167	82	1.782
Open Space	168 - 169		0.826
3.00m Walkways	170 - 171		0.034
S.W.M. Pond	172		0.794
Environmental Protection	173		0.862
0.3m Reserves	174 - 175		0.005
Future Road Connection	176		0.050
Roads			3.513
TOTAL	176	236	13.575

IPS INNOVATIVE PLANNING SOLUTIONS
PLANNERS • PROJECT MANAGERS • LAND DEVELOPERS
647 WELHAM ROAD, UNIT 9, BARRIE, ON, L4N 0B7
tel: 705 • 812 • 3281 fax: 705 • 812 • 3438 e: info@ipsconsultinginc.com www.ipsconsultinginc.com

Date:	March 11, 2024	Drawn By:	A.S.
File:	21 - 1241	Checked:	K.B.

Appendix B: Existing Conditions Analysis

PROJECT	Maple Bridge Subdivision, Phase 2	FILE	422492		
		DATE	10/10/2024		
SUBJECT	VO Schematic - Pre- Development - AMCII & AMCIII	NAME	LJC		
		PAGE	1	OF	1



NASHYD



ROUTE PIPE



DUHYD



STANDHYD



ROUTE CHANNEL



DIVERT HYD



ADDHYD



ROUTE RESERVOIR

Visual OTTHYMO Model Parameter Calculations (NasHYD)

Project Details

Maple Bridge Subdivision	422492
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Data Sources

Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997)

Prepared By

LJC	10/2/2024
-----	-----------

Pre-Development Condition

Watershed:	LSRCA
Catchment ID:	100
Catchment Area (ha):	0.62
Impervious %:	

Average Curve Number (CN), Runoff Coefficient (C) and Initial Abstraction (IA)

Soil Symbol		Ds											
Soil Series		Dundonald											
Hydrologic Soils Group		AB											
Soil Texture		Sand Loam											
Runoff Coefficient Type		1											
Area (ha)		0.62											
Percentage of Catchment		100%											
Land Cover Category	IA	A (ha)	CN	C	A (ha)	CN	C	A (ha)	CN	C	A (ha)	CN	C
Impervious	2		98	0.95									
Gravel	3		81	0.14									
Woodland	10	0.25	46	0.12									
Pasture/Lawns	5		59	0.15									
Meadows	8	0.12	51	0.14									
Cultivated	7	0.25	68	0.30									
Waterbody	12		50	0.05									
Average CN		55.74											
Average C		0.19											
Average IA		8.42											

Time to Peak Calculations

Max. Catchment Elev. (m):	286.24
Min. Catchment Elev. (m):	284.05
Catchment Length (m):	29
Catchment Slope (%):	7.55%
Method:	Airport Method
Time of Concentration (mins):	8.16

Summary

Catchment CN:	55.7
Catchment C:	0.19
Catchment IA (mm):	8.42
Time of Concentration (hrs):	0.14
Catchment Time to Peak (hrs):	0.09
Catchment Time Step (mins):	1.09

Visual OTTHYMO Model Parameter Calculations (NasHYD)

Project Details

Maple Bridge Subdivision	422492
--------------------------	--------

Data Sources

Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997)

Prepared By

LJC	10/2/2024
-----	-----------

Pre-Development Condition

Watershed:	LSRCA
Catchment ID:	101
Catchment Area (ha):	13.85
Impervious %:	1%

Average Curve Number (CN), Runoff Coefficient (C) and Initial Abstraction (IA)

Soil Symbol		Ds				Brs							
Soil Series		Dundonald				Brighton							
Hydrologic Soils Group		AB				A							
Soil Texture		Sand Loam				Sand							
Runoff Coefficient Type		1				1							
Area (ha)		9.89				3.97							
Percentage of Catchment		71%				29%							
Land Cover Category	IA	A (ha)	CN	C	A (ha)	CN	C	A (ha)	CN	C	A (ha)	CN	C
Impervious	2	0.11	98	0.95		98	0.95						
Gravel	3		81	0.09		76	0.09						
Woodland	10	0.90	46	0.08	0.70	32	0.08						
Pasture/Lawns	5	0.95	59	0.10		49	0.10						
Meadows	8	1.82	51	0.09	0.51	38	0.09						
Cultivated	7	6.11	68	0.22	2.76	62	0.22						
Waterbody	12		50	0.05		50	0.05						
Average CN		62.33				53.64							
Average C		0.18				0.18							
Average IA		7.21				7.66							

Time to Peak Calculations

Max. Catchment Elev. (m):	287.50
Min. Catchment Elev. (m):	266.66
Catchment Length (m):	697
Catchment Slope (%):	2.99%
Method:	Airport Method
Time of Concentration (mins):	55.20

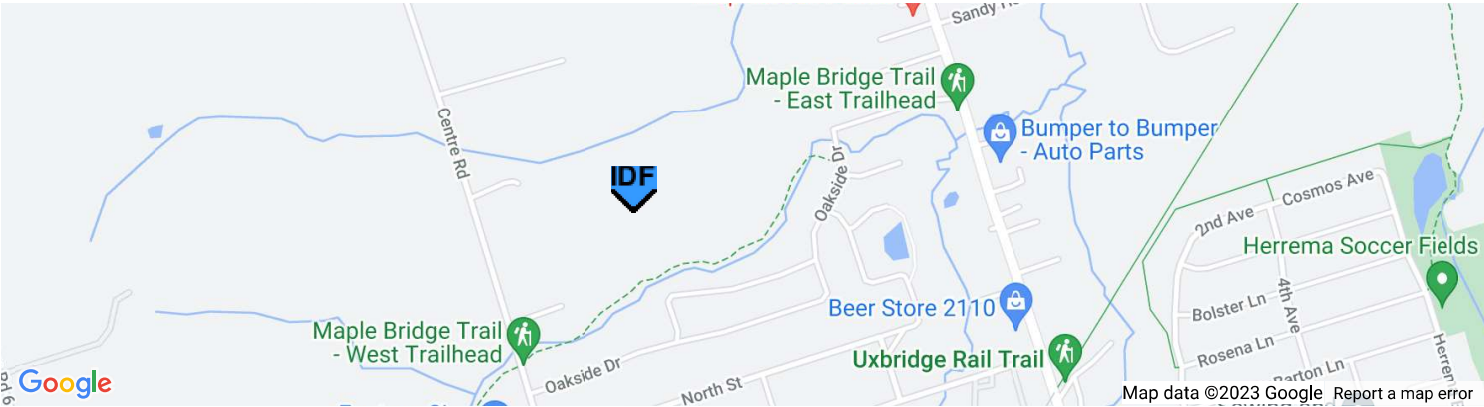
Summary

Catchment CN:	59.8
Catchment C:	0.18
Catchment IA (mm):	7.34
Time of Concentration (hrs):	0.92
Catchment Time to Peak (hrs):	0.61
Catchment Time Step (mins):	7.36

Active coordinate

44° 7' 15" N, 79° 7' 44" W (44.120833,-79.129167)

Retrieved: Mon, 14 Aug 2023 12:52:26 GMT



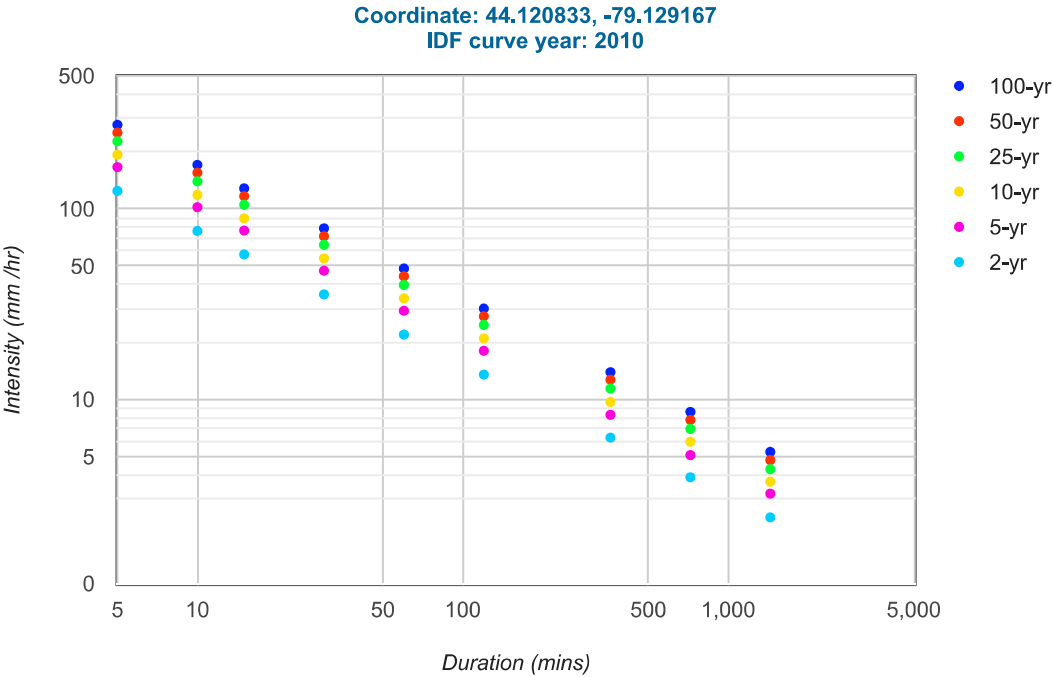
Location summary

These are the locations in the selection.

IDF Curve: 44° 7' 15" N, 79° 7' 44" W (44.120833,-79.129167)

Results

An IDF curve was found.



Coefficient summary

IDF Curve: 44° 7' 15" N, 79° 7' 44" W (44.120833,-79.129167)

Retrieved: Mon, 14 Aug 2023 12:52:26 GMT

Data year: 2010

IDF curve year: 2010

Return period	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr
A	21.9	29.2	33.9	39.9	44.3	48.7
B	-0.699	-0.699	-0.699	-0.699	-0.699	-0.699

Statistics

Rainfall intensity (mm hr⁻¹)

Duration	5-min	10-min	15-min	30-min	1-hr	2-hr	6-hr	12-hr	24-hr
2-yr	124.4	76.6	57.7	35.6	21.9	13.5	6.3	3.9	2.4
5-yr	165.9	102.2	77.0	47.4	29.2	18.0	8.3	5.1	3.2
10-yr	192.6	118.6	89.3	55.0	33.9	20.9	9.7	6.0	3.7
25-yr	226.6	139.6	105.2	64.8	39.9	24.6	11.4	7.0	4.3
50-yr	251.6	155.0	116.7	71.9	44.3	27.3	12.7	7.8	4.8
100-yr	276.6	170.4	128.3	79.1	48.7	30.0	13.9	8.6	5.3

Rainfall depth (mm)

Duration	5-min	10-min	15-min	30-min	1-hr	2-hr	6-hr	12-hr	24-hr
2-yr	10.4	12.8	14.4	17.8	21.9	27.0	37.6	46.3	57.0
5-yr	13.8	17.0	19.2	23.7	29.2	36.0	50.1	61.7	76.0
10-yr	16.0	19.8	22.3	27.5	33.9	41.8	58.1	71.6	88.2
25-yr	18.9	23.3	26.3	32.4	39.9	49.2	68.4	84.3	103.9
50-yr	21.0	25.8	29.2	36.0	44.3	54.6	76.0	93.6	115.3
100-yr	23.1	28.4	32.1	39.5	48.7	60.0	83.5	102.9	126.8

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V V I SS U U A A L
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O O T T H H Y Y M M O O
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USER:

COMMENTS: _____

** SIMULATION : 25 mm 4-hr CHI **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

CHIC STORM [Ptot= 25.00 mm]	10.0								
* ** CALIB NASHYD [CN=59.8 [N = 3.0:Tp 0.61]	0101	1	5.0	13.85	0.03	2.58	1.65	0.07	0.000
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FINISH									

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USER:

COMMENTS: _____

** SIMULATION : Run 01 - 2yr 4hr 10min Chicag **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

CHIC STORM [Ptot= 33.23 mm]	10.0								
* ** CALIB NASHYD [CN=59.8 [N = 3.0:Tp 0.61]	0101	1	5.0	13.85	0.06	2.25	3.41	0.10	0.000
* ** CALIB NASHYD [CN=55.7 [N = 3.0:Tp 0.09]	0100	1	5.0	0.62	0.01	1.33	2.61	0.08	0.000
FINISH									

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V V I SS U U A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

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COMMENTS: _____

** SIMULATION : Run 02 - 5yr 4hr 10min Chicag **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

CHIC STORM [Ptot= 44.30 mm]	10.0								
* ** CALIB NASHYD [CN=59.8 [N = 3.0:Tp 0.61]	0101	1	5.0	13.85	0.06	2.25	3.41	0.10	0.000
* ** CALIB NASHYD [CN=55.7 [N = 3.0:Tp 0.09]	0100	1	5.0	0.62	0.01	1.33	2.61	0.08	0.000
FINISH									

=====

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** CALIB NASHYD      0101  1  5.0   13.85    0.11  2.17   6.58 0.15   0.000
  [CN=59.8
  [ N = 3.0:Tp 0.61]
*
  CHIC STORM          10.0
  [ Ptot= 44.30 mm ]
*
** CALIB NASHYD      0100  1  5.0    0.62    0.01  1.33   5.21 0.12   0.000
  [CN=55.7
  [ N = 3.0:Tp 0.09]
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DATE: 10/02/2024 TIME: 01:35:49

USER:

COMMENTS: _____

** SIMULATION : Run 03 - 10yr 4hr 10min Chica **

```
W/E COMMAND          HYD ID  DT    AREA  '  Qpeak Tpeak  R.V. R.C.  Qbase
                        min    ha    '    cms  hrs   mm   mm   cms

  START @ 0.00 hrs
  -----
  CHIC STORM          10.0
  [ Ptot= 51.44 mm ]
*
** CALIB NASHYD      0101  1  5.0   13.85    0.16  2.17   9.05 0.18   0.000
  [CN=59.8
  [ N = 3.0:Tp 0.61]
*
  CHIC STORM          10.0
  [ Ptot= 51.44 mm ]
*
** CALIB NASHYD      0100  1  5.0    0.62    0.02  1.33   7.27 0.14   0.000
  [CN=55.7
  [ N = 3.0:Tp 0.09]
*
=====
```

```

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000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
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O  O  T  T  H  H  Y  Y  M  M  O  O
000  T  T  H  H  Y  Y  M  M  000

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USER:

COMMENTS: _____

** SIMULATION : Run 04 - 25yr 4hr 10min Chica **

```
W/E COMMAND          HYD ID  DT    AREA  '  Qpeak Tpeak  R.V. R.C.  Qbase
                        min    ha    '    cms  hrs   mm   mm   cms

  START @ 0.00 hrs
  -----
  CHIC STORM          10.0
  [ Ptot= 60.54 mm ]
*
** CALIB NASHYD      0101  1  5.0   13.85    0.23  2.08  12.64 0.21   0.000
  [CN=59.8
  [ N = 3.0:Tp 0.61]
*
  CHIC STORM          10.0
  [ Ptot= 60.54 mm ]
*
** CALIB NASHYD      0100  1  5.0    0.62    0.03  1.33  10.28 0.17   0.000
  [CN=55.7
  [ N = 3.0:Tp 0.09]
*
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```

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V  V  I  SS    U  U  A  A  L
VV   I  SSSSS  UUUUU  A  A  LLLLL

000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
O  O  T  T  H  H  Y  Y  MM MM  O  O
O  O  T  T  H  H  Y  Y  M  M  O  O
000  T  T  H  H  Y  Y  M  M  000

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```

***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\95107345-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\95107345-

DATE: 10/02/2024 TIME: 01:35:49

USER:

COMMENTS: _____

** SIMULATION : Run 05 - 50yr 4hr 10min Chica **

```
W/E COMMAND          HYD ID  DT    AREA  '  Qpeak Tpeak  R.V. R.C.  Qbase
                        min    ha    '    cms  hrs   mm   mm   cms

  START @ 0.00 hrs
  -----
```

```

CHIC STORM              10.0
[ Ptot= 67.21 mm ]

*
** CALIB NASHYD          0101  1  5.0   13.85   0.28  2.08  15.54  0.23   0.000
[CN=59.8
[ N = 3.0:Tp 0.61]

*
CHIC STORM              10.0
[ Ptot= 67.21 mm ]

*
** CALIB NASHYD          0100  1  5.0    0.62   0.04  1.33  12.75  0.19   0.000
[CN=55.7
[ N = 3.0:Tp 0.09]

*
=====
```

```

V  V  I  SSSSS  U  U  A  L              (v 6.2.2015)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
V  V  I  SS    U  U  A  A  L
VV    I  SSSSS  UUUUU A  A  LLLLL
```

```

000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
O  O  T  T  H  H  Y  Y  MM MM  O  O
O  O  T  T  H  H  Y  Y  M  M  O  O
000  T  T  H  H  Y  Y  M  M  000
```

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***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\1548b85d-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\1548b85d-

DATE: 10/02/2024 TIME: 01:35:49

USER:

COMMENTS: _____

** SIMULATION : Run 06 - 100yr 4hr 10min Chic **

```

W/E COMMAND          HYD ID  DT  AREA  '  Qpeak Tpeak  R.V.  R.C.  Qbase
                        min   ha   '   cms   hrs   mm      cms

START @ 0.00 hrs
-----
CHIC STORM              10.0
[ Ptot= 73.89 mm ]

*
** CALIB NASHYD          0101  1  5.0   13.85   0.34  2.08  18.66  0.25   0.000
[CN=59.8
[ N = 3.0:Tp 0.61]

*
CHIC STORM              10.0
[ Ptot= 73.89 mm ]

*
** CALIB NASHYD          0100  1  5.0    0.62   0.04  1.33  15.42  0.21   0.000
[CN=55.7
[ N = 3.0:Tp 0.09]

*
=====
```

```

V  V  I  SSSSS  U  U  A  L              (v 6.2.2015)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
V  V  I  SS    U  U  A  A  L
VV    I  SSSSS  UUUUU A  A  LLLLL
```

```

000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
O  O  T  T  H  H  Y  Y  MM MM  O  O
O  O  T  T  H  H  Y  Y  M  M  O  O
000  T  T  H  H  Y  Y  M  M  000
```

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***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\6973c860-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\6973c860-

DATE: 10/02/2024

TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : Run 07 - 2yr 12hr 15min SCS T **

```

W/E COMMAND          HYD ID  DT  AREA  '  Qpeak Tpeak  R.V.  R.C.  Qbase
                        min   ha   '   cms   hrs   mm      cms

START @ 0.00 hrs
-----
READ STORM              15.0
[ Ptot= 46.27 mm ]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\b2ae71f7-ae0f-442a-b969
remark: 2yr 12hr 15min SCS Type II (MTO)

*
** CALIB NASHYD          0101  1  5.0   13.85   0.13  6.83   7.23  0.16   0.000
[CN=59.8
[ N = 3.0:Tp 0.61]

*
READ STORM              15.0
[ Ptot= 46.27 mm ]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\b2ae71f7-ae0f-442a-b969
remark: 2yr 12hr 15min SCS Type II (MTO)

*
** CALIB NASHYD          0100  1  5.0    0.62   0.02  6.25   5.75  0.12   0.000
[CN=55.7
[ N = 3.0:Tp 0.09]

*
=====
```

```

V  V  I  SSSSS  U  U  A  L              (v 6.2.2015)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
V  V  I  SS    U  U  A  A  L
VV    I  SSSSS  UUUUU A  A  LLLLL
```

```

000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
O  O  T  T  H  H  Y  Y  MM MM  O  O
O  O  T  T  H  H  Y  Y  M  M  O  O
000  T  T  H  H  Y  Y  M  M  000
```

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***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\1744a2a2-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\1744a2a2-

DATE: 10/02/2024

TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : Run 08 - 5yr 12hr 15min SCS T **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								
[Ptot= 61.69 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\12dc4c30-dc5c-4036-a369									
remark: 5yr 12hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0101	1	5.0	13.85	0.24	6.83	13.12	0.21	0.000
[CN=59.8]									
[N = 3.0:Tp 0.61]									

READ STORM	15.0								
[Ptot= 61.69 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\12dc4c30-dc5c-4036-a369									
remark: 5yr 12hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0100	1	5.0	0.62	0.03	6.25	10.70	0.17	0.000
[CN=55.7]									
[N = 3.0:Tp 0.09]									

V	V	I	SSSSS	U	U	A	L		(v 6.2.2015)
V	V	I	SS	U	U	A	A	L	
V	V	I	SS	U	U	AAAAA	L		
V	V	I	SS	U	U	A	A	L	
VV	I	SSSSS	UUUUU	A	A	LLLLL			

000	TTTTT	TTTTT	H	H	Y	Y	M	M	000	TM	
O	O	T	T	H	H	Y	Y	MM	MM	O	O
O	O	T	T	H	H	Y	Y	M	M	O	O
000	T	T	H	H	Y	Y	M	M	000		

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\cdc9f2c3-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\cdc9f2c3-

DATE: 10/02/2024

TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : Run 09 - 10yr 12hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								
[Ptot= 71.62 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\9eae1c74-c47d-4044-bc76									
remark: 10yr 12hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0101	1	5.0	13.85	0.32	6.83	17.58	0.25	0.000
[CN=59.8]									
[N = 3.0:Tp 0.61]									

READ STORM	15.0								
[Ptot= 71.62 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\9eae1c74-c47d-4044-bc76									
remark: 10yr 12hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0100	1	5.0	0.62	0.04	6.25	14.49	0.20	0.000
[CN=55.7]									

[N = 3.0:Tp 0.09]

*

V	V	I	SSSSS	U	U	A	L		(v 6.2.2015)
V	V	I	SS	U	U	A	A	L	
V	V	I	SS	U	U	AAAAA	L		
V	V	I	SS	U	U	A	A	L	
VV	I	SSSSS	UUUUU	A	A	LLLLL			

000	TTTTT	TTTTT	H	H	Y	Y	M	M	000	TM	
O	O	T	T	H	H	Y	Y	MM	MM	O	O
O	O	T	T	H	H	Y	Y	M	M	O	O
000	T	T	H	H	Y	Y	M	M	000		

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\8616212a-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\8616212a-

DATE: 10/02/2024

TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : Run 10 - 25yr 12hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								
[Ptot= 84.30 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\973c359e-7963-43c5-9f84									
remark: 25yr 12hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0101	1	5.0	13.85	0.44	6.83	23.91	0.28	0.000
[CN=59.8]									
[N = 3.0:Tp 0.61]									

READ STORM	15.0								
[Ptot= 84.30 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\973c359e-7963-43c5-9f84									
remark: 25yr 12hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0100	1	5.0	0.62	0.05	6.25	19.94	0.24	0.000
[CN=55.7]									
[N = 3.0:Tp 0.09]									

V	V	I	SSSSS	U	U	A	L		(v 6.2.2015)
V	V	I	SS	U	U	A	A	L	
V	V	I	SS	U	U	AAAAA	L		
V	V	I	SS	U	U	A	A	L	
VV	I	SSSSS	UUUUU	A	A	LLLLL			

000	TTTTT	TTTTT	H	H	Y	Y	M	M	000	TM	
O	O	T	T	H	H	Y	Y	MM	MM	O	O
O	O	T	T	H	H	Y	Y	M	M	O	O
000	T	T	H	H	Y	Y	M	M	000		

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voim.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\1624ff89-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\1624ff89-

DATE: 10/02/2024 TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : Run 11 - 50yr 12hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	' '	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								
[Ptot= 93.59 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\42fd7725-35a6-48e7-a68b									
remark: 50yr 12hr 15min SCS Type II (MTO)									

** CALIB NASHYD	0101	1	5.0	13.85	0.53	6.83	28.95	0.31	0.000
[CN=59.8]									
[N = 3.0:Tp 0.61]									

READ STORM	15.0								
[Ptot= 93.59 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\42fd7725-35a6-48e7-a68b									
remark: 50yr 12hr 15min SCS Type II (MTO)									

** CALIB NASHYD	0100	1	5.0	0.62	0.07	6.25	24.31	0.26	0.000
[CN=55.7]									
[N = 3.0:Tp 0.09]									

V V I SSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

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***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voim.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\b3009718-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\b3009718-

DATE: 10/02/2024 TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : Run 12 - 100yr 12hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	' '	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								

[Ptot=102.89 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\b9bc5393-c514-4b42-b02e
remark: 100yr 12hr 15min SCS Type II (MTO)

* ** CALIB NASHYD 0101 1 5.0 13.85 0.63 6.83 34.28 0.33 0.000
[CN=59.8]
[N = 3.0:Tp 0.61]

* READ STORM 15.0
[Ptot=102.89 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\b9bc5393-c514-4b42-b02e
remark: 100yr 12hr 15min SCS Type II (MTO)

* ** CALIB NASHYD 0100 1 5.0 0.62 0.08 6.25 28.96 0.28 0.000
[CN=55.7]
[N = 3.0:Tp 0.09]

V V I SSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

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***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voim.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\397eef87-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\397eef87-

DATE: 10/02/2024 TIME: 01:35:49

USER:

COMMENTS: _____

** SIMULATION : Run 13 - 2yr 24hr 15min SCS T **

W/E COMMAND	HYD ID	DT min	AREA ha	' '	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
-------------	--------	--------	---------	-----	-----------	-----------	---------	------	-----------

START @ 0.00 hrs

READ STORM 15.0
[Ptot= 57.00 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\2b6e45fc-8f7a-4339-92f1
remark: 2yr 24hr 15min SCS Type II (MTO)

* ** CALIB NASHYD 0101 1 5.0 13.85 0.17 12.83 11.19 0.20 0.000
[CN=59.8]
[N = 3.0:Tp 0.61]

* READ STORM 15.0
[Ptot= 57.00 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\2b6e45fc-8f7a-4339-92f1
remark: 2yr 24hr 15min SCS Type II (MTO)

* ** CALIB NASHYD 0100 1 5.0 0.62 0.02 12.25 9.06 0.16 0.000
[CN=55.7]
[N = 3.0:Tp 0.09]

V V I SSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A A L

V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\6e584004-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\6e584004-

DATE: 10/02/2024 TIME: 01:35:49

USER:

COMMENTS: _____

** SIMULATION : Run 14 - 5yr 24hr 15min SCS T **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								
[Ptot= 76.00 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\2b0170f9-a76f-44e2-b9a1									
remark: 5yr 24hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0101	1	5.0	13.85	0.30	12.83	19.69	0.26	0.000
[CN=59.8									
[N = 3.0:Tp 0.61]									
* ** READ STORM	15.0								
[Ptot= 76.00 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\2b0170f9-a76f-44e2-b9a1									
remark: 5yr 24hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0100	1	5.0	0.62	0.04	12.25	16.30	0.21	0.000
[CN=55.7									
[N = 3.0:Tp 0.09]									

V V I SSSS U U A A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\84d41a3c-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\84d41a3c-

DATE: 10/02/2024 TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : Run 15 - 10yr 24hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								
[Ptot= 88.24 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\922067fb-cd2e-49ce-b832									
remark: 10yr 24hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0101	1	5.0	13.85	0.40	12.83	26.01	0.29	0.000
[CN=59.8									
[N = 3.0:Tp 0.61]									
* ** READ STORM	15.0								
[Ptot= 88.24 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\922067fb-cd2e-49ce-b832									
remark: 10yr 24hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0100	1	5.0	0.62	0.05	12.25	21.75	0.25	0.000
[CN=55.7									
[N = 3.0:Tp 0.09]									

V V I SSSS U U A A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\9289e100-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\9289e100-

DATE: 10/02/2024 TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : Run 16 - 25yr 24hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								
[Ptot=103.85 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\5887a2ec-b7af-4120-8fc6									
remark: 25yr 24hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0101	1	5.0	13.85	0.54	12.75	34.85	0.34	0.000
[CN=59.8									
[N = 3.0:Tp 0.61]									

READ STORM 15.0
[Ptot=103.85 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\5887a2ec-b7af-4120-8fc6
remark: 25yr 24hr 15min SCS Type II (MTO)

* ** CALIB NASHYD 0100 1 5.0 0.62 0.07 12.25 29.46 0.28 0.000
[CN=55.7]
[N = 3.0:Tp 0.09]
*

=====

V V I SSSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
OOO T T H H Y M M OOO

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\995a0e26-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\995a0e26-

DATE: 10/02/2024 TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : Run 17 - 50yr 24hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	' '	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
-------------	--------	-----------	------------	--------	--------------	--------------	------------	------	--------------

START @ 0.00 hrs

READ STORM 15.0
[Ptot=115.30 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\8f729958-6162-4208-9eed
remark: 50yr 24hr 15min SCS Type II (MTO)

* ** CALIB NASHYD 0101 1 5.0 13.85 0.66 12.75 41.82 0.36 0.000
[CN=59.8]
[N = 3.0:Tp 0.61]
*

READ STORM 15.0
[Ptot=115.30 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\8f729958-6162-4208-9eed
remark: 50yr 24hr 15min SCS Type II (MTO)

* ** CALIB NASHYD 0100 1 5.0 0.62 0.09 12.25 35.59 0.31 0.000
[CN=55.7]
[N = 3.0:Tp 0.09]
*

=====

V V I SSSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
OOO T T H H Y M M OOO

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\1eca5525-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\1eca5525-

DATE: 10/02/2024

TIME: 01:35:49

USER:

COMMENTS: _____

** SIMULATION : Run 18 - 100yr 24hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	' '	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
-------------	--------	-----------	------------	--------	--------------	--------------	------------	------	--------------

START @ 0.00 hrs

READ STORM 15.0
[Ptot=126.76 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\4335f593-3a45-4748-8bfa
remark: 100yr 24hr 15min SCS Type II (MTO)

* ** CALIB NASHYD 0101 1 5.0 13.85 0.78 12.75 49.15 0.39 0.000
[CN=59.8]
[N = 3.0:Tp 0.61]
*

READ STORM 15.0
[Ptot=126.76 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\4335f593-3a45-4748-8bfa
remark: 100yr 24hr 15min SCS Type II (MTO)

* ** CALIB NASHYD 0100 1 5.0 0.62 0.10 12.25 42.06 0.33 0.000
[CN=55.7]
[N = 3.0:Tp 0.09]
*

V V I SSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000
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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voim.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\0685eb48-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\0685eb48-

DATE: 10/02/2024 TIME: 01:35:50

USER:

COMMENTS: _____

** SIMULATION : 25 mm 4-hr CHI **

READ STORM		Filename: C:\Users\lcarretas\AppData\Local\Temp\c9ece19a-4254-4647-8b68-1317744a1847\d98d1005					
Ptotal= 25.00 mm		Comments: 25 mm 4-hr CHI					
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.00	2.07	1.00	5.70	2.00	5.19	3.00	2.80
0.17	2.27	1.17	10.78	2.17	4.47	3.17	2.62
0.33	2.52	1.33	50.21	2.33	3.95	3.33	2.48
0.50	2.88	1.50	13.37	2.50	3.56	3.50	2.35
0.67	3.38	1.67	8.29	2.67	3.25	3.67	2.23
0.83	4.18	1.83	6.30	2.83	3.01	3.83	2.14

CALIB NASHYD (0101) ID= 1 DT= 5.0 min	Area (ha)= 13.85 Ia (mm)= 7.34 U.H. Tp(hrs)= 0.61	Curve Number (CN)= 59.8 # of Linear Res.(N)= 3.00
---	---	--

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.07	1.083	5.70	2.083	5.19	3.08	2.80
0.167	2.07	1.167	5.70	2.167	5.19	3.17	2.80
0.250	2.27	1.250	10.78	2.250	4.47	3.25	2.62
0.333	2.27	1.333	10.78	2.333	4.47	3.33	2.62
0.417	2.52	1.417	50.21	2.417	3.95	3.42	2.48
0.500	2.52	1.500	50.21	2.500	3.95	3.50	2.48
0.583	2.88	1.583	13.37	2.583	3.56	3.58	2.35
0.667	2.88	1.667	13.37	2.667	3.56	3.67	2.35
0.750	3.38	1.750	8.29	2.750	3.25	3.75	2.23
0.833	3.38	1.833	8.29	2.833	3.25	3.83	2.23
0.917	4.17	1.917	6.30	2.917	3.01	3.92	2.14
1.000	4.18	2.000	6.29	3.000	3.01	4.00	2.14

Unit Hyd Qpeak (cms)= 0.867

PEAK FLOW (cms)= 0.026 (i)
TIME TO PEAK (hrs)= 2.583
RUNOFF VOLUME (mm)= 1.654
TOTAL RAINFALL (mm)= 24.996
RUNOFF COEFFICIENT = 0.066

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| Junction Command(0901) |

INFLOW : ID= 2(0101)	AREA (ha) 13.85	QPEAK (cms) 0.03	TPEAK (hrs) 2.58	R.V. (mm) 1.65
OUTFLOW: ID= 2(0901)	13.85	0.03	2.58	1.65

CALIB NASHYD (0100) ID= 1 DT= 5.0 min	Area (ha)= 0.62 Ia (mm)= 8.42 U.H. Tp(hrs)= 0.09	Curve Number (CN)= 55.7 # of Linear Res.(N)= 3.00
---	--	--

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.07	1.083	5.70	2.083	5.19	3.08	2.80
0.167	2.07	1.167	5.70	2.167	5.19	3.17	2.80
0.250	2.27	1.250	10.78	2.250	4.47	3.25	2.62
0.333	2.27	1.333	10.78	2.333	4.47	3.33	2.62
0.417	2.52	1.417	50.21	2.417	3.95	3.42	2.48
0.500	2.52	1.500	50.21	2.500	3.95	3.50	2.48
0.583	2.88	1.583	13.37	2.583	3.56	3.58	2.35
0.667	2.88	1.667	13.37	2.667	3.56	3.67	2.35
0.750	3.38	1.750	8.29	2.750	3.25	3.75	2.23
0.833	3.38	1.833	8.29	2.833	3.25	3.83	2.23
0.917	4.17	1.917	6.30	2.917	3.01	3.92	2.14
1.000	4.18	2.000	6.29	3.000	3.01	4.00	2.14

Unit Hyd Qpeak (cms)= 0.263

PEAK FLOW (cms)= 0.002 (i)
TIME TO PEAK (hrs)= 1.583
RUNOFF VOLUME (mm)= 1.209
TOTAL RAINFALL (mm)= 24.996
RUNOFF COEFFICIENT = 0.048

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

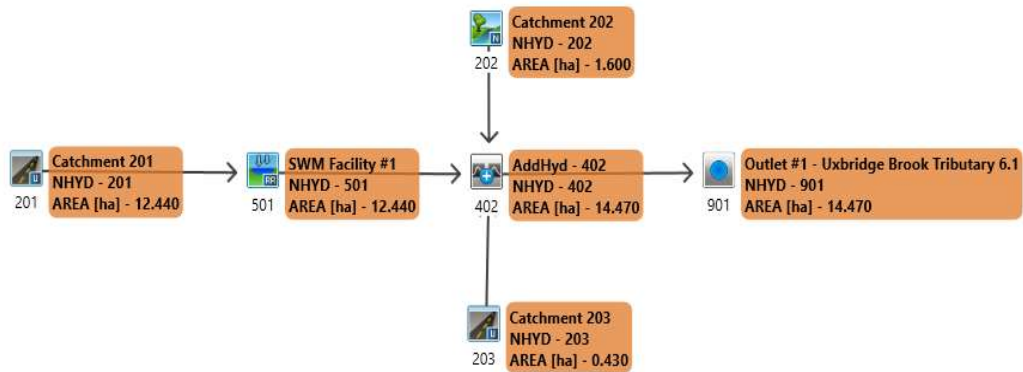
| Junction Command(0902) |

INFLOW : ID= 2(0100)	AREA (ha) 0.62	QPEAK (cms) 0.00	TPEAK (hrs) 1.58	R.V. (mm) 1.21
OUTFLOW: ID= 2(0902)	0.62	0.00	1.58	1.21

FINISH

Appendix C: Proposed Condition Analysis

PROJECT	Maple Bridge Subdivision, Phase 2	FILE	422492
		DATE	10/10/2024
SUBJECT	VO Schematic - Post- Development - AMCII & AMCIII	NAME	LJC
		PAGE	1 OF 1



NASHYD



ROUTE PIPE



DUHYD



STANDHYD



ROUTE CHANNEL



DIVERT HYD

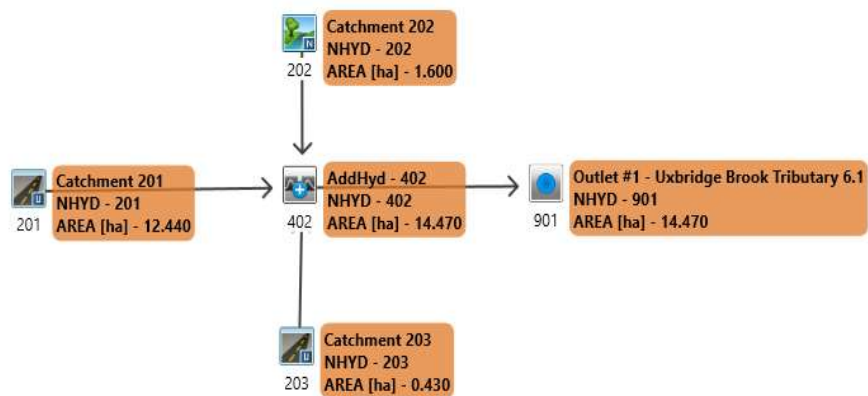


ADDHYD



ROUTE RESERVOIR

PROJECT	Maple Bridge Subdivision, Phase 2	FILE	422492
		DATE	10/10/2024
SUBJECT	VO Schematic - Post- Development - AMCI, No Pond	NAME	LJC
		PAGE	1 OF 1



NASHYD



ROUTE PIPE



DUHYD



STANDHYD



ROUTE CHANNEL



DIVERT HYD



ADDHYD



ROUTE RESERVOIR

Visual OTTHYMO Model Parameter Calculations (NasHYD)

Project Details

Maple Bridge Subdivision	422492
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Data Sources

Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997)

Prepared By

LJC	10/2/2024
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Post Development Condition

Watershed:	LSRCA
Catchment ID:	202
Catchment Area (ha):	1.60
Impervious %:	1%

Average Curve Number (CN), Runoff Coefficient (C) and Initial Abstraction (IA)

Soil Symbol		Ds				Brs							
Soil Series		Dundonald				Brighton							
Hydrologic Soils Group		AB				A							
Soil Texture		Sand Loam				Sand							
Runoff Coefficient Type		1				1							
Area (ha)		1.28				0.32							
Percentage of Catchment		80%				20%							
Land Cover Category	IA	A (ha)	CN	C	A (ha)	CN	C	A (ha)	CN	C	A (ha)	CN	C
Impervious	2	0.02	98	0.95		98	0.95						
Gravel	3		81	0.09		76	0.09						
Woodland	10	0.60	46	0.08	0.26	32	0.08						
Pasture/Lawns	5	0.66	59	0.10	0.06	49	0.10						
Meadows	8		51	0.09		38	0.09						
Cultivated	7		68	0.22		62	0.22						
Waterbody	12		50	0.05		50	0.05						
Average CN		53.52				35.19							
Average C		0.10				0.08							
Average IA		7.30				9.06							

Time to Peak Calculations

Max. Catchment Elev. (m):	284.91
Min. Catchment Elev. (m):	284.40
Catchment Length (m):	25.3
Catchment Slope (%):	2.02%
Method: Airport Method	
Time of Concentration (mins):	13.01

Summary

Catchment CN:	49.9
Catchment C:	0.10
Catchment IA (mm):	7.65
Time of Concentration (hrs):	0.22
Catchment Time to Peak (hrs):	0.14
Catchment Time Step (mins):	1.74

Visual OTTHYMO Model Parameter Calculations (StandHYD)

Project Details

Maple Bridge Subdivision	422492
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Data Sources

Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997)

Prepared By

LJC	10/2/2024
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Post Development Condition

Watershed:	LSRCA
Catchment ID:	201
Catchment Area (ha):	12.44
Impervious %:	71%
Pervious Area (ha):	3.55

Average Curve Number (CN) and Initial Abstraction (IA) for Pervious Area

Soil Symbol		Ds		Brs					
Soil Series		Dundonald		Brighton					
Hydrologic Soils Group		AB		A					
Soil Texture		Sand Loam		Sand					
Runoff Coefficient Type		1		1					
Area (ha)		2.48		1.06					
Percentage of Catchment		70%		30%					
Land Cover Category	IA	A (ha)	CN	A (ha)	CN	A (ha)	CN	A (ha)	CN
Impervious	2		98		98				
Gravel	3		81		76				
Woodland	10		46		32				
Pasture/Lawns	5	2.48	59	1.06	49				
Meadows	8		51		38				
Cultivated	7		68		62				
Waterbody	12		50		50				
Average CN		59.00		49.00					
Average IA		5.00		5.00					

Notes

CN and IA values have been calculated for the pervious area of the catchment only.

Summary

Catchment CN:	56.0
Catchment IA (mm):	5.00

Visual OTTHYMO Model Parameter Calculations (StandHYD)

Project Details

Maple Bridge Subdivision	422492
--------------------------	--------

Data Sources

Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997)

Prepared By

LJC	10/2/2024
-----	-----------

Post Development Condition

Watershed:	LSRCA
Catchment ID:	203
Catchment Area (ha):	0.43
Impervious %:	47%
Pervious Area (ha):	0.23

Average Curve Number (CN) and Initial Abstraction (IA) for Pervious Area

Soil Symbol		Brs							
Soil Series		Brighton							
Hydrologic Soils Group		A							
Soil Texture		Sand							
Runoff Coefficient Type		1							
Area (ha)		0.23							
Percentage of Catchment		100%							
Land Cover Category	IA	A (ha)	CN	A (ha)	CN	A (ha)	CN	A (ha)	CN
Impervious	2		98						
Gravel	3		76						
Woodland	10		32						
Pasture/Lawns	5	0.23	49						
Meadows	8		38						
Cultivated	7		62						
Waterbody	12		50						
Average CN		49.00							
Average IA		5.00							

Notes

CN and IA values have been calculated for the pervious area of the catchment only.

Summary

Catchment CN:	49.0
Catchment IA (mm):	5.00

PROJECT	Maple Bridge Subdivision, Phase 2	FILE	422492	
		DATE	10/4/2024	
SUBJECT	Land Use Allocation - Post- Development	NAME	LJC	
		PAGE	1	OF 1

Catchment 201 - StandHYD								
Land Use Category	Area	Total Imperviousness (TIMP)	TIMP Area	Directly Connected Imperviousness (XIMP)	XIMP Area	Pervious Area	Pervious CN	Pervious IA
	ha	%	ha	%	ha	ha	-	mm
22' Courtyard	1.79	82%	1.47	55%	0.81	0.32	56.0	5.00
36' Classic	4.57	67%	3.06	27%	0.83	1.51	56.0	5.00
46' Classic	0.55	61%	0.34	21%	0.07	0.21	56.0	5.00
Catch. 202 Impervious	0.37	100%	0.37	14%	0.05	0.00	56.0	5.00
Holdout Residential	0.91	100%	0.91	67%	0.61	0.00	56.0	5.00
Road Allowances	3.26	74%	2.40	74%	1.77	0.86	56.0	5.00
SWMF#1	0.35	100%	0.35	100%	0.35	0.00	56.0	5.00
Landscape	0.64	0%	0.00	0%	0.00	0.64	56.0	5.00
Total	12.44	71%	8.89	36%	4.48	3.55	56.0	5.00

Catchment 202 - NasHYD								
Land Use Category	Area	Total Imperviousness (TIMP)	TIMP Area	Directly Connected Imperviousness (XIMP)	XIMP Area	Pervious Area	CN	IA
	ha	%	ha	%	ha	ha	-	mm
Walkways	0.02	100%	0.02	100%	0.02	0.00	98.0	2.00
Woodland	0.86	0%	0.00	0%	0.00	0.86	46.0	10.00
Landscape	0.72	0%	0.00	0%	0.00	0.72	59.0	5.00
Total	1.60	1%	0.02	1%	0.02	1.58	49.9	7.65

Catchment 203 - StandHYD								
Land Use Category	Area	Total Imperviousness (TIMP)	TIMP Area	Directly Connected Imperviousness (XIMP)	XIMP Area	Pervious Area	Pervious CN	Pervious IA
	ha	%	ha	%	ha	ha	-	mm
Road Allowances	0.29	70%	0.20	70%	0.14	0.09	49.0	5.00
Landscape	0.14	0%	0.00	0%	0.00	0.14	49.0	5.00
Total	0.43	47%	0.20	33%	0.14	0.23	49.0	5.00

Summary

Total Site Area:	14.47 ha
Total Site TIMP Area:	9.11 ha
Total Site TIMP:	63 %

PROJECT	Maple Bridge Subdivision, Phase 2	FILE	422492
		DATE	10/17/2024
SUBJECT	Road Allowance Imperviousness	NAME	LJC
		PAGE	1 OF 1

Catchment 201

Pavement, Sidewalk, and Driveway Area

Street Name	Area of ROW (m ²)	Length of Road (m)	Road Paved Area (m ²)	Sidewalk Area (m ²)	Driveway Area in ROW (m ²)	Total Imp. Area (m ²)	Total Imp. %
Street A (20 m)	11,841	590	6,490	1,770	828	9,088	76.8%
Street B (17 m)	2,249	132	1,175	198	376	1,749	77.7%
Street C (17 m)	2,103	123	1,095	185	317	1,596	75.9%
Street D (17 m)	3,137	150	1,335	225	593	2,153	68.6%
Street E (17 m)	5,152	286	2,545	429	890	3,865	75.0%
Street F (17 m)	5,972	292	2,599	438	890	3,927	65.8%
Lane A (7.5 m)	1,245	132	726	0	120	846	67.9%
Lane B (7.5 m)	924	123	677	0	96	773	83.6%
Total	32,624	1,828	16,641	3,245	4,110	23,996	73.6%

Catchment 203

Pavement, Sidewalk, and Driveway Area

Street Name	Area of ROW (m ²)	Length of Road (m)	Road Paved Area (m ²)	Sidewalk Area (m ²)	Driveway Area in ROW (m ²)	Total Imp. Area (m ²)	Total Imp. %
Street A (20 m)	2,923	146	1,608	439	0	2,047	70.0%
Total	2,923	146	1,608	439	0	2,047	70.0%

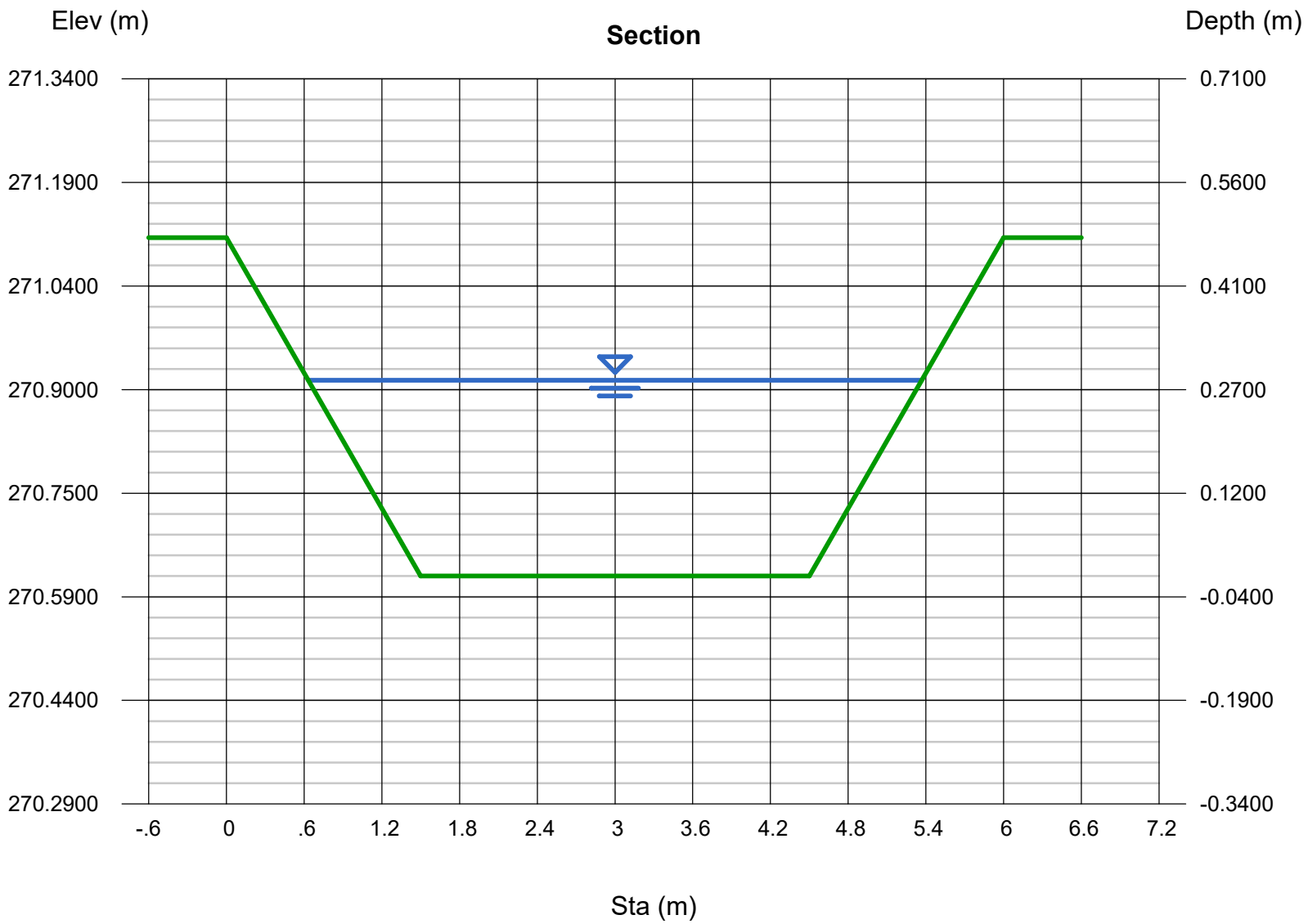
Notes

1. OPSP 600.070 was utilized to determine the width of curb.

Channel Report

Drainage Channel to SWM Facility - Hazel (Uncontrolled)

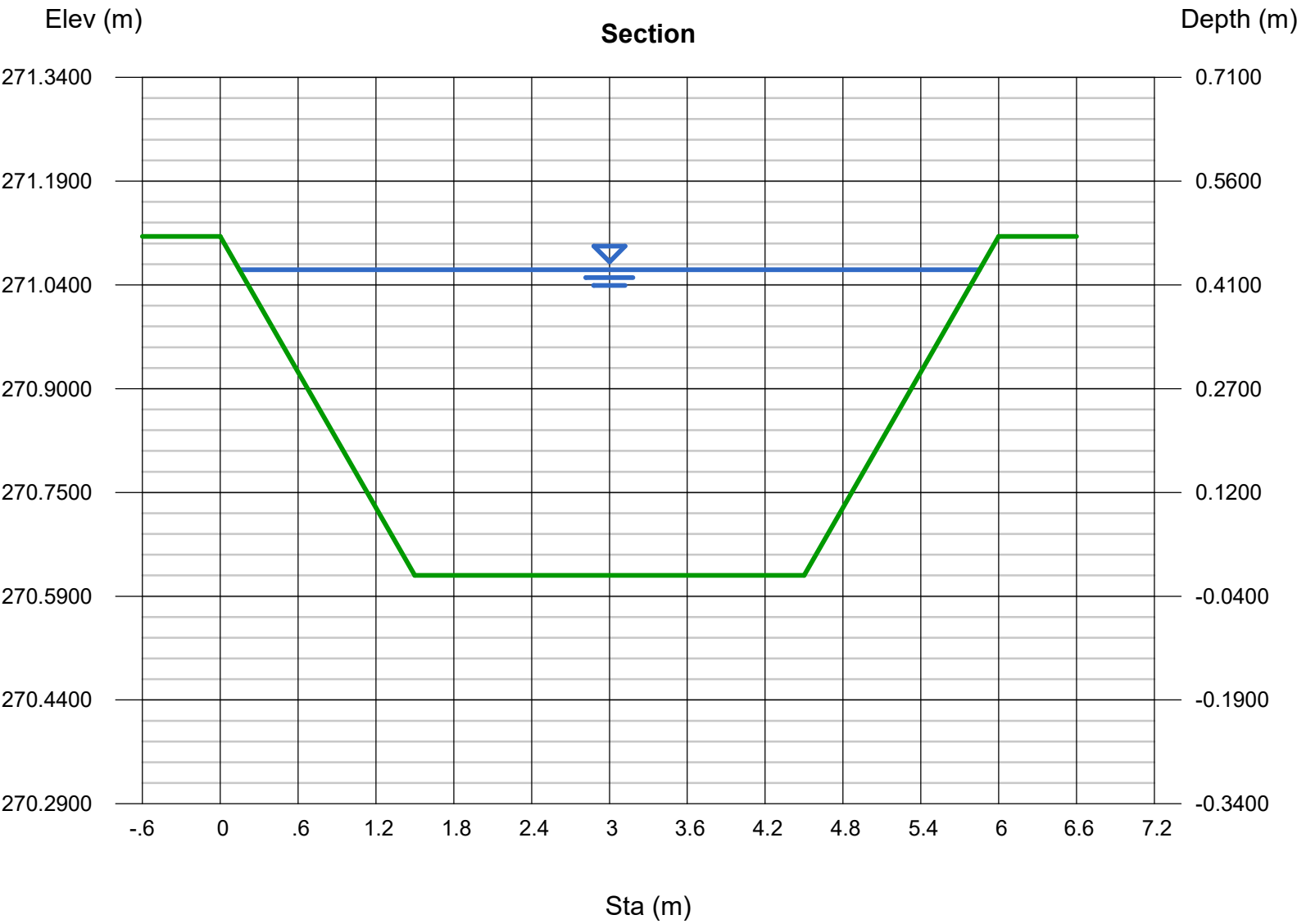
User-defined		Highlighted	
Invert Elev (m)	= 270.6300	Depth (m)	= 0.2835
Slope (%)	= 1.3000	Q (cms)	= 1.7650
N-Value	= 0.026	Area (sqm)	= 1.0964
		Velocity (m/s)	= 1.6099
		Wetted Perim (m)	= 4.8257
Calculations		Crit Depth, Yc (m)	= 0.2957
Compute by:	Known Q	Top Width (m)	= 4.7355
Known Q (cms)	= 1.7650	EGL (m)	= 0.4157
(Sta, El, n)-(Sta, El, n)...			
(0.0000, 271.1200)-(1.5000, 270.6300, 0.040)-(4.5000, 270.6300, 0.016)-(6.0000, 271.1200, 0.040)			



Channel Report

Drainage Channel to SWM Facility - 1:100-Year 24-hr SCS Type II (Uncontrolled)

User-defined		Highlighted	
Invert Elev (m)	= 270.6300	Depth (m)	= 0.4420
Slope (%)	= 1.3000	Q (cms)	= 3.5950
N-Value	= 0.029	Area (sqm)	= 1.9238
Calculations		Velocity (m/s)	= 1.8686
Compute by:	Known Q	Wetted Perim (m)	= 5.8466
Known Q (cms)	= 3.5950	Crit Depth, Yc (m)	= 0.4511
		Top Width (m)	= 5.7059
		EGL (m)	= 0.6201
(Sta, El, n)-(Sta, El, n)...			
(0.0000, 271.1200)-(1.5000, 270.6300, 0.040)-(4.5000, 270.6300, 0.016)-(6.0000, 271.1200, 0.040)			



=====

V V I SSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\c6362252-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\c6362252-

DATE: 10/21/2024 TIME: 09:49:19

USER:

COMMENTS: _____

** SIMULATION : 25 mm 4-hr CHI **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									
READ STORM [Ptot= 25.00 mm] fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\d98d1005-1e85-4b79-95a8 remark: 25 mm 4-hr CHI	10.0								
** CALIB NASHYD [CN=49.9 [N = 3.0:Tp 0.14]	0202	1 5.0	1.60	0.00	1.67	1.10	0.04	0.000	
* * READ STORM [Ptot= 25.00 mm] fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\d98d1005-1e85-4b79-95a8 remark: 25 mm 4-hr CHI	10.0								
** CALIB STANDHYD [I%=36.0:S%= 2.00]	0201	1 5.0	12.44	0.54	1.50	11.56	0.46	0.000	
** Reservoir OUTFLOW:	0501	1 5.0	12.44	0.02	2.50	9.07	n/a	0.000	
* * READ STORM [Ptot= 25.00 mm] fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\d98d1005-1e85-4b79-95a8 remark: 25 mm 4-hr CHI	10.0								
* CALIB STANDHYD [I%=33.0:S%= 2.00]	0203	1 5.0	0.43	0.02	1.50	9.16	0.37	0.000	
* ADD [0202+ 0203]	0402	3 5.0	2.03	0.02	1.50	2.80	n/a	0.000	
* ADD [0402+ 0501]	0402	1 5.0	14.47	0.02	2.50	8.19	n/a	0.000	
* *****									

V V I SSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\52c640dc-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\52c640dc-

DATE: 10/21/2024 TIME: 09:49:17

USER:

COMMENTS: _____

** SIMULATION : Run 01 - 2yr 4hr 10min Chicag **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									
CHIC STORM [Ptot= 33.23 mm]	10.0								
* ** CALIB NASHYD [CN=49.9 [N = 3.0:Tp 0.14]	0202	1 5.0	1.60	0.01	1.42	2.31	0.07	0.000	
* * CHIC STORM [Ptot= 33.23 mm]	10.0								
** CALIB STANDHYD [I%=36.0:S%= 2.00]	0201	1 5.0	12.44	0.88	1.33	16.66	0.50	0.000	
** Reservoir OUTFLOW:	0501	1 5.0	12.44	0.02	1.58	14.16	n/a	0.000	
* * CHIC STORM [Ptot= 33.23 mm]	10.0								
* CALIB STANDHYD [I%=33.0:S%= 2.00]	0203	1 5.0	0.43	0.03	1.33	13.01	0.39	0.000	
* ADD [0202+ 0203]	0402	3 5.0	2.03	0.04	1.33	4.58	n/a	0.000	
* ADD [0402+ 0501]	0402	1 5.0	14.47	0.04	1.33	12.82	n/a	0.000	
* *****									

V V I SSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\fe27ed00-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\fe27ed00-

DATE: 10/21/2024 TIME: 09:49:19

USER:

COMMENTS: _____

** SIMULATION : Run 02 - 5yr 4hr 10min Chicag **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									
CHIC STORM [Ptot= 44.30 mm]									
* ** CALIB NASHYD [CN=49.9 [N = 3.0:Tp 0.14]	0202	1	5.0	1.60	0.02	1.42	4.57	0.10	0.000
* ** CALIB STANDHYD [I%=36.0:S%= 2.00]	0201	1	5.0	12.44	1.29	1.33	24.13	0.54	0.000
* ** Reservoir OUTFLOW:	0501	1	5.0	12.44	0.04	4.17	21.63	n/a	0.000
* CHIC STORM [Ptot= 44.30 mm]									
* ** CALIB STANDHYD [I%=33.0:S%= 2.00]	0203	1	5.0	0.43	0.04	1.33	18.64	0.42	0.000
* ADD [0202+ 0203]	0402	3	5.0	2.03	0.06	1.33	7.55	n/a	0.000
* ADD [0402+ 0501]	0402	1	5.0	14.47	0.07	1.33	19.66	n/a	0.000
FINISH									

=====

V V I SSSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
OOO T T H H Y M M OOO

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\2a9d34a8-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\2a9d34a8-

DATE: 10/21/2024 TIME: 09:49:16

USER:

COMMENTS: _____

** SIMULATION : Run 03 - 10yr 4hr 10min Chica **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									
CHIC STORM [Ptot= 51.44 mm]									
* ** CALIB NASHYD [CN=49.9 [N = 3.0:Tp 0.14]	0202	1	5.0	1.60	0.03	1.42	6.37	0.12	0.000
* CHIC STORM [Ptot= 51.44 mm]									
* ** CALIB STANDHYD [I%=36.0:S%= 2.00]	0201	1	5.0	12.44	1.55	1.33	29.24	0.57	0.000
* ** Reservoir OUTFLOW:	0501	1	5.0	12.44	0.06	4.08	26.75	n/a	0.000
* CHIC STORM [Ptot= 51.44 mm]									
* ** CALIB STANDHYD [I%=33.0:S%= 2.00]	0203	1	5.0	0.43	0.05	1.33	22.51	0.44	0.000
* ADD [0202+ 0203]	0402	3	5.0	2.03	0.07	1.33	9.79	n/a	0.000
* ADD [0402+ 0501]	0402	1	5.0	14.47	0.09	1.33	24.37	n/a	0.000

=====

V V I SSSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
OOO T T H H Y M M OOO

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\93deb9a5-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\93deb9a5-

DATE: 10/21/2024 TIME: 09:49:18

USER:

COMMENTS: _____

** SIMULATION : Run 04 - 25yr 4hr 10min Chica **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									
CHIC STORM [Ptot= 60.54 mm]									
* ** CALIB NASHYD [CN=49.9 [N = 3.0:Tp 0.14]	0202	1	5.0	1.60	0.05	1.42	9.02	0.15	0.000
* ** CALIB STANDHYD [I%=36.0:S%= 2.00]	0201	1	5.0	12.44	1.55	1.33	29.24	0.57	0.000

```

CHIC STORM          10.0
[ Ptot= 60.54 mm ]
*
* CALIB STANDHYD    0201  1  5.0   12.44   1.90  1.33  36.05 0.60   0.000
[I%=36.0:S%= 2.00]
*
** Reservoir
OUTFLOW:           0501  1  5.0   12.44   0.12  4.00  33.56 n/a   0.000
*
CHIC STORM          10.0
[ Ptot= 60.54 mm ]
*
* CALIB STANDHYD    0203  1  5.0    0.43   0.06  1.33  27.70 0.46   0.000
[I%=33.0:S%= 2.00]
*
ADD [ 0202+ 0203]  0402  3  5.0    2.03   0.10  1.33  12.97 n/a   0.000
*
ADD [ 0402+ 0501]  0402  1  5.0   14.47   0.13  4.00  30.67 n/a   0.000
*
=====

```

```

V  V  I  SSSSS  U  U  A  L          (v 6.2.2015)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
VV   I  SSSSS  UUUUU  A  A  LLLLL

000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
O  O  T    T    T  H  H  Y  Y  MM MM  O  O
O  O  T    T    T  H  H  Y    M  M  O  O
000  T    T    T  H  H  Y    M  M  000

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```

***** S U M M A R Y O U T P U T *****

```

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\7001a54e-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\7001a54e-

```

DATE: 10/21/2024 TIME: 09:49:17

USER:

COMMENTS: _____

```

*****
** SIMULATION : Run 05 - 50yr 4hr 10min Chica **
*****

```

W/E COMMAND	HYD ID	DT min	AREA ha	' '	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

CHIC STORM [Ptot= 67.21 mm]			10.0						
* ** CALIB NASHYD [CN=49.9 [N = 3.0:Tp 0.14]	0202	1 5.0	1.60	0.06	1.42	11.19	0.17	0.000	
* CHIC STORM [Ptot= 67.21 mm]			10.0						
* ** CALIB STANDHYD [I%=36.0:S%= 2.00]	0201	1 5.0	12.44	2.17	1.33	41.21	0.61	0.000	
* ** Reservoir OUTFLOW:	0501	1 5.0	12.44	0.16	3.42	38.72	n/a	0.000	
* CHIC STORM [Ptot= 67.21 mm]			10.0						
* ** CALIB STANDHYD [I%=33.0:S%= 2.00]	0203	1 5.0	0.43	0.07	1.33	31.67	0.47	0.000	

```

ADD [ 0202+ 0203]  0402  3  5.0    2.03   0.12  1.33  15.53 n/a   0.000
*
ADD [ 0402+ 0501]  0402  1  5.0   14.47   0.18  3.33  35.47 n/a   0.000
*
=====

```

```

V  V  I  SSSSS  U  U  A  L          (v 6.2.2015)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
VV   I  SSSSS  UUUUU  A  A  LLLLL

000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
O  O  T    T    T  H  H  Y  Y  MM MM  O  O
O  O  T    T    T  H  H  Y    M  M  O  O
000  T    T    T  H  H  Y    M  M  000

```

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***** S U M M A R Y O U T P U T *****

```

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\8d4025d6-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\8d4025d6-

```

DATE: 10/21/2024

TIME: 09:49:18

USER:

COMMENTS: _____

```

*****
** SIMULATION : Run 06 - 100yr 4hr 10min Chic **
*****

```

W/E COMMAND	HYD ID	DT min	AREA ha	' '	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

CHIC STORM [Ptot= 73.89 mm]			10.0						
* ** CALIB NASHYD [CN=49.9 [N = 3.0:Tp 0.14]	0202	1 5.0	1.60	0.07	1.42	13.56	0.18	0.000	
* CHIC STORM [Ptot= 73.89 mm]			10.0						
* ** CALIB STANDHYD [I%=36.0:S%= 2.00]	0201	1 5.0	12.44	2.72	1.33	46.50	0.63	0.000	
* ** Reservoir OUTFLOW:	0501	1 5.0	12.44	0.23	2.83	44.01	n/a	0.000	
* CHIC STORM [Ptot= 73.89 mm]			10.0						
* ** CALIB STANDHYD [I%=33.0:S%= 2.00]	0203	1 5.0	0.43	0.07	1.33	35.78	0.48	0.000	
* ADD [0202+ 0203]	0402	3 5.0	2.03	0.14	1.33	18.26	n/a	0.000	
* ADD [0402+ 0501]	0402	1 5.0	14.47	0.25	2.83	40.40	n/a	0.000	

```

V  V  I  SSSSS  U  U  A  L          (v 6.2.2015)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
VV   I  SSSSS  UUUUU  A  A  LLLLL

000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
O  O  T    T    T  H  H  Y  Y  MM MM  O  O

```

USER:

COMMENTS: _____

** SIMULATION : Run 09 - 10yr 12hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM		15.0							
[Ptot= 71.62 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\9eae1c74-c47d-4044-bc76									
remark: 10yr 12hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0202	1	5.0	1.60	0.07	6.25	12.73	0.18	0.000
[CN=49.9									
[N = 3.0:Tp 0.14]									

READ STORM		15.0							
[Ptot= 71.62 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\9eae1c74-c47d-4044-bc76									
remark: 10yr 12hr 15min SCS Type II (MTO)									
* ** CALIB STANDHYD	0201	1	5.0	12.44	1.73	6.25	44.69	0.62	0.000
[I%=36.0:S%= 2.00]									
* ** Reservoir									
OUTFLOW:									
0501	1	5.0	12.44	0.15	7.33	42.20	n/a	0.000	

READ STORM		15.0							
[Ptot= 71.62 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\9eae1c74-c47d-4044-bc76									
remark: 10yr 12hr 15min SCS Type II (MTO)									
* ** CALIB STANDHYD	0203	1	5.0	0.43	0.05	6.25	34.37	0.48	0.000
[I%=33.0:S%= 2.00]									
* ** ADD [0202+ 0203]	0402	3	5.0	2.03	0.12	6.25	17.32	n/a	0.000
* ** ADD [0402+ 0501]	0402	1	5.0	14.47	0.16	7.25	38.71	n/a	0.000

=====

V	V	I	SSSS	U	U	A	L		(v 6.2.2015)
V	V	I	SS	U	U	A	A	L	
V	V	I	SS	U	U	AAAA	L		
V	V	I	SS	U	U	A	A	L	
VV		I	SSSS	UUUU	A	A	LLLL		

000	TTTT	TTTT	H	H	Y	Y	M	M	000	TM	
O	O	T	T	H	H	Y	Y	MM	MM	O	O
O	O	T	T	H	H	Y	M	M	O	O	
000		T	H	H	Y	M	M	O	000		

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\2e1a344e-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\2e1a344e-

DATE: 10/21/2024 TIME: 09:49:16

USER:

COMMENTS: _____

** SIMULATION : Run 10 - 25yr 12hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM		15.0							
[Ptot= 84.30 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\973c359e-7963-43c5-9f84									
remark: 25yr 12hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0202	1	5.0	1.60	0.10	6.25	17.58	0.21	0.000
[CN=49.9									
[N = 3.0:Tp 0.14]									

READ STORM		15.0							
[Ptot= 84.30 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\973c359e-7963-43c5-9f84									
remark: 25yr 12hr 15min SCS Type II (MTO)									
* ** CALIB STANDHYD	0201	1	5.0	12.44	2.16	6.25	54.96	0.65	0.000
[I%=36.0:S%= 2.00]									
* ** Reservoir									
OUTFLOW:									
0501	1	5.0	12.44	0.31	6.92	52.47	n/a	0.000	

READ STORM		15.0							
[Ptot= 84.30 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\973c359e-7963-43c5-9f84									
remark: 25yr 12hr 15min SCS Type II (MTO)									
* ** CALIB STANDHYD	0203	1	5.0	0.43	0.06	6.25	42.41	0.50	0.000
[I%=33.0:S%= 2.00]									
* ** ADD [0202+ 0203]	0402	3	5.0	2.03	0.16	6.25	22.84	n/a	0.000
* ** ADD [0402+ 0501]	0402	1	5.0	14.47	0.34	6.83	48.31	n/a	0.000

=====

V	V	I	SSSS	U	U	A	L		(v 6.2.2015)
V	V	I	SS	U	U	A	A	L	
V	V	I	SS	U	U	AAAA	L		
V	V	I	SS	U	U	A	A	L	
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000		T	H	H	Y	M	M	O	000		

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\45b462a5-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\45b462a5-

DATE: 10/21/2024 TIME: 09:49:17

USER:

COMMENTS: _____

** SIMULATION : Run 11 - 50yr 12hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM		15.0							
[Ptot= 93.59 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\42fd7725-35a6-48e7-a68b									
remark: 50yr 12hr 15min SCS Type II (MTO)									

** CALIB NASHYD 0202 1 5.0 1.60 0.12 6.25 21.50 0.23 0.000
[CN=49.9
[N = 3.0:Tp 0.14]
*
READ STORM 15.0
[Ptot= 93.59 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\42fd7725-35a6-48e7-a68b
remark: 50yr 12hr 15min SCS Type II (MTO)
*
* CALIB STANDHYD 0201 1 5.0 12.44 2.48 6.25 62.70 0.67 0.000
[I%=36.0:S%= 2.00]
** Reservoir
OUTFLOW: 0501 1 5.0 12.44 0.34 6.92 60.21 n/a 0.000
*
READ STORM 15.0
[Ptot= 93.59 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\42fd7725-35a6-48e7-a68b
remark: 50yr 12hr 15min SCS Type II (MTO)
*
* CALIB STANDHYD 0203 1 5.0 0.43 0.07 6.25 48.56 0.52 0.000
[I%=33.0:S%= 2.00]
*
ADD [0202+ 0203] 0402 3 5.0 2.03 0.19 6.25 27.23 n/a 0.000
*
ADD [0402+ 0501] 0402 1 5.0 14.47 0.40 6.50 55.58 n/a 0.000
*
=====

V V I SSSS U U A L (v 6.2.2015)
V V I SS U U A A L
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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\66529810-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\66529810-

DATE: 10/21/2024 TIME: 09:49:17

USER:

COMMENTS:

** SIMULATION : Run 12 - 100yr 12hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	' cms	Qpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs								
READ STORM [Ptot=102.89 mm] fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\b9bc5393-c514-4b42-b02e remark: 100yr 12hr 15min SCS Type II (MTO)	15.0							
** CALIB NASHYD [CN=49.9 [N = 3.0:Tp 0.14]	0202 1 5.0		1.60	0.14	6.25	25.70	0.25	0.000
* READ STORM [Ptot=102.89 mm] fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\b9bc5393-c514-4b42-b02e remark: 100yr 12hr 15min SCS Type II (MTO)	15.0							

*
* CALIB STANDHYD 0201 1 5.0 12.44 3.03 6.25 70.60 0.69 0.000
[I%=36.0:S%= 2.00]
** Reservoir
OUTFLOW: 0501 1 5.0 12.44 0.35 6.92 68.10 n/a 0.000
*
READ STORM 15.0
[Ptot=102.89 mm]
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\b9bc5393-c514-4b42-b02e
remark: 100yr 12hr 15min SCS Type II (MTO)
*
* CALIB STANDHYD 0203 1 5.0 0.43 0.08 6.25 54.89 0.53 0.000
[I%=33.0:S%= 2.00]
*
ADD [0202+ 0203] 0402 3 5.0 2.03 0.23 6.25 31.89 n/a 0.000
*
ADD [0402+ 0501] 0402 1 5.0 14.47 0.49 6.33 63.02 n/a 0.000
*
=====

V V I SSSS U U A L (v 6.2.2015)
V V I SS U U A A L
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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\9767455b-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\9767455b-

DATE: 10/21/2024 TIME: 09:49:18

USER:

COMMENTS:

** SIMULATION : Run 13 - 2yr 24hr 15min SCS T **

W/E COMMAND	HYD ID	DT min	AREA ha	' cms	Qpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs								
READ STORM [Ptot= 57.00 mm] fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\2b6e45fc-8f7a-4339-92f1 remark: 2yr 24hr 15min SCS Type II (MTO)	15.0							
** CALIB NASHYD [CN=49.9 [N = 3.0:Tp 0.14]	0202 1 5.0		1.60	0.04	12.25	7.94	0.14	0.000
* READ STORM [Ptot= 57.00 mm] fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\2b6e45fc-8f7a-4339-92f1 remark: 2yr 24hr 15min SCS Type II (MTO)	15.0							
* ** CALIB STANDHYD [I%=36.0:S%= 2.00]	0201 1 5.0		12.44	1.14	12.25	33.37	0.59	0.000
* ** Reservoir OUTFLOW: 0501 1 5.0	12.44		0.04	14.67	30.87	n/a		0.000
* READ STORM [Ptot= 57.00 mm]	15.0							

fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\2b6e45fc-8f7a-4339-92f1
remark: 2yr 24hr 15min SCS Type II (MTO)

* CALIB STANDHYD 0203 1 5.0 0.43 0.03 12.25 25.65 0.45 0.000
[I%=33.0:S%= 2.00]

* ADD [0202+ 0203] 0402 3 5.0 2.03 0.07 12.25 11.69 n/a 0.000

* ADD [0402+ 0501] 0402 1 5.0 14.47 0.09 12.25 28.18 n/a 0.000

*

V V I SSSSS U U A L (v 6.2.2015)
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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voim.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\7c31591f-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\7c31591f-

DATE: 10/21/2024 TIME: 09:49:17

USER:

COMMENTS: _____

** SIMULATION : Run 14 - 5yr 24hr 15min SCS T **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM			15.0						
[Ptot= 76.00 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\2b0170f9-a76f-44e2-b9a1									
remark: 5yr 24hr 15min SCS Type II (MTO)									
** CALIB NASHYD	0202	1	5.0	1.60	0.07	12.25	14.34	0.19	0.000
[CN=49.9									
[N = 3.0:Tp 0.14]									
* READ STORM			15.0						
[Ptot= 76.00 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\2b0170f9-a76f-44e2-b9a1									
remark: 5yr 24hr 15min SCS Type II (MTO)									
* CALIB STANDHYD	0201	1	5.0	12.44	1.68	12.25	48.19	0.63	0.000
[I%=36.0:S%= 2.00]									
** Reservoir									
OUTFLOW:	0501	1	5.0	12.44	0.13	13.42	45.70	n/a	0.000
* READ STORM			15.0						
[Ptot= 76.00 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\2b0170f9-a76f-44e2-b9a1									
remark: 5yr 24hr 15min SCS Type II (MTO)									
* CALIB STANDHYD	0203	1	5.0	0.43	0.05	12.25	37.10	0.49	0.000
[I%=33.0:S%= 2.00]									
* ADD [0202+ 0203]	0402	3	5.0	2.03	0.12	12.25	19.16	n/a	0.000
* ADD [0402+ 0501]	0402	1	5.0	14.47	0.14	13.33	41.98	n/a	0.000

*

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V V I SSSSS U U A L (v 6.2.2015)
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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voim.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\e6cd0312-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\e6cd0312-

DATE: 10/21/2024 TIME: 09:49:19

USER:

COMMENTS: _____

** SIMULATION : Run 15 - 10yr 24hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM			15.0						
[Ptot= 88.24 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\922067fb-cd2e-49ce-b832									
remark: 10yr 24hr 15min SCS Type II (MTO)									
** CALIB NASHYD	0202	1	5.0	1.60	0.09	12.25	19.21	0.22	0.000
[CN=49.9									
[N = 3.0:Tp 0.14]									
* READ STORM			15.0						
[Ptot= 88.24 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\922067fb-cd2e-49ce-b832									
remark: 10yr 24hr 15min SCS Type II (MTO)									
* CALIB STANDHYD	0201	1	5.0	12.44	2.05	12.25	58.22	0.66	0.000
[I%=36.0:S%= 2.00]									
** Reservoir									
OUTFLOW:	0501	1	5.0	12.44	0.26	13.00	55.73	n/a	0.000
* READ STORM			15.0						
[Ptot= 88.24 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\922067fb-cd2e-49ce-b832									
remark: 10yr 24hr 15min SCS Type II (MTO)									
* CALIB STANDHYD	0203	1	5.0	0.43	0.06	12.25	44.99	0.51	0.000
[I%=33.0:S%= 2.00]									
* ADD [0202+ 0203]	0402	3	5.0	2.03	0.15	12.25	24.67	n/a	0.000
* ADD [0402+ 0501]	0402	1	5.0	14.47	0.28	12.92	51.37	n/a	0.000

V V I SSSSS U U A L (v 6.2.2015)
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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\d54e4c18-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\d54e4c18-

DATE: 10/21/2024 TIME: 09:49:19

USER:

COMMENTS: _____

** SIMULATION : Run 16 - 25yr 24hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								
[Ptot=103.85 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\5887a2ec-b7af-4120-8fc6									
remark: 25yr 24hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0202	1	5.0	1.60	0.13	12.25	26.15	0.25	0.000
[CN=49.9									
[N = 3.0:Tp 0.14]									

READ STORM	15.0								
[Ptot=103.85 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\5887a2ec-b7af-4120-8fc6									
remark: 25yr 24hr 15min SCS Type II (MTO)									
* ** CALIB STANDHYD	0201	1	5.0	12.44	2.55	12.25	71.42	0.69	0.000
[I%=36.0:S%= 2.00]									
* ** Reservoir									
OUTFLOW:									
0501	1	5.0	12.44	0.34	12.92	68.93	n/a	0.000	

READ STORM	15.0								
[Ptot=103.85 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\5887a2ec-b7af-4120-8fc6									
remark: 25yr 24hr 15min SCS Type II (MTO)									
* ** CALIB STANDHYD	0203	1	5.0	0.43	0.07	12.25	55.56	0.53	0.000
[I%=33.0:S%= 2.00]									
* ADD [0202+ 0203]	0402	3	5.0	2.03	0.20	12.25	32.38	n/a	0.000
* ADD [0402+ 0501]	0402	1	5.0	14.47	0.41	12.42	63.80	n/a	0.000

V V I SSSS U U A L (v 6.2.2015)
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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\ac03dacf-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\ac03dacf-

DATE: 10/21/2024 TIME: 09:49:18

USER:

COMMENTS: _____

** SIMULATION : Run 17 - 50yr 24hr 15min SCS **

W/E COMMAND	HYD ID	DT min	AREA ha	'	Qpeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs									

READ STORM	15.0								
[Ptot=115.30 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\8f729958-6162-4208-9eed									
remark: 50yr 24hr 15min SCS Type II (MTO)									
* ** CALIB NASHYD	0202	1	5.0	1.60	0.16	12.25	31.71	0.28	0.000
[CN=49.9									
[N = 3.0:Tp 0.14]									

READ STORM	15.0								
[Ptot=115.30 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\8f729958-6162-4208-9eed									
remark: 50yr 24hr 15min SCS Type II (MTO)									
* ** CALIB STANDHYD	0201	1	5.0	12.44	3.17	12.25	81.34	0.71	0.000
[I%=36.0:S%= 2.00]									
* ** Reservoir									
OUTFLOW:									
0501	1	5.0	12.44	0.36	12.92	78.84	n/a	0.000	

READ STORM	15.0								
[Ptot=115.30 mm]									
fname : C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\8f729958-6162-4208-9eed									
remark: 50yr 24hr 15min SCS Type II (MTO)									
* ** CALIB STANDHYD	0203	1	5.0	0.43	0.09	12.25	63.63	0.55	0.000
[I%=33.0:S%= 2.00]									
* ADD [0202+ 0203]	0402	3	5.0	2.03	0.24	12.25	38.47	n/a	0.000
* ADD [0402+ 0501]	0402	1	5.0	14.47	0.51	12.33	73.18	n/a	0.000

V V I SSSS U U A L (v 6.2.2015)
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VV I SSSS UUUU A A LLLLL

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***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\4048e7d1-
Summary filename: C:\Users\lcarretas\AppData\Local\Civica\XH5\13981d5a-de9d-475d-ae56-06a5922f796b\4048e7d1-

TIME: 09:49:16

COMMENTS: _____

W/E COMMAND	HYD ID	DT min	AREA ha	Qpeak ' cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
-------------	--------	-----------	------------	-------------------	--------------	------------	------	--------------

START @ 0.00 hrs

```

[ Ptot=126.76 mm ]
fname : C:\Users\carretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\4335f593-3a45-4748-8bfa
remark: 100yr 24hr 15min SCS Type II (MTO)

```

READ STORM	15.0
------------	------

```

name : C:\Users\carretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\4335f593-3a45-4748-8bfa
remark: 100yr 24hr 15min SCS Type II (MTO)

```

** Reservoir									
OUTFLOW:	0501	1	5.0	12.44	0.38	12.92	88.93	n/a	0.000

```
[ Ptot=126.76 mm ]
fname : C:\Users\jcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\4335f593-3a45-4748-8bfa
remark: 100yr 24hr 15min SCS Type II (MTO)
```

ADD	[	0202+	0203]	0402	3	5.0	2.03	0.29	12.25	44.90	n/a	0.000
-----	---	-------	-------	------	---	-----	------	------	-------	-------	-----	-------

ADD	[	0402+	0501]	0402	1	5.0	14.47	0.61	12.25	82.76	n/a	0.000
-----	---	-------	-------	------	---	-----	-------	------	-------	-------	-----	-------

* ADD [0402+ 0301] 0402 1 3.0 14.47 0.61 12.23 82.76 11/a 0.000

V V I SSSSS U U A L (v 6.2.2015)
V V I SS U U A A L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000
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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voim.dat
Output filename: C:\Users\lcarretas\AppData\Local\Civilica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\c6362252-
Summary filename: C:\Users\lcarretas\AppData\Local\Civilica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\c6362252-

DATE: 10/21/2024 TIME: 09:49:19

USER:

COMMENTS: _____

** SIMULATION : 25 mm 4-hr CHI **

READ STORM	Filename: C:\Users\lcarretas\AppData\Local\Temp\d6c13652-22bd-4428-b9a5-33f68710d380\d98d1005
Ptotal= 25.00 mm	Comments: 25 mm 4-hr CHI

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	2.07	1.00	5.70	2.00	5.19	3.00	2.80
0.17	2.27	1.17	10.78	2.17	4.47	3.17	2.62
0.33	2.52	1.33	50.21	2.33	3.95	3.33	2.48
0.50	2.88	1.50	13.37	2.50	3.56	3.50	2.35
0.67	3.38	1.67	8.29	2.67	3.25	3.67	2.23
0.83	4.18	1.83	6.30	2.83	3.01	3.83	2.14

CALIB	NASHYD (0202)	Area (ha)= 1.60	Curve Number (CN)= 49.9
ID= 1 DT= 5.0 min	Ia (mm)= 7.65	# of Linear Res.(N)= 3.00	
	U.H. Tp(hrs)= 0.14		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.07	1.083	5.70	2.083	5.19	3.08	2.80
0.167	2.07	1.167	5.70	2.167	5.19	3.17	2.80
0.250	2.27	1.250	10.78	2.250	4.47	3.25	2.62
0.333	2.27	1.333	10.78	2.333	4.47	3.33	2.62
0.417	2.52	1.417	50.21	2.417	3.95	3.42	2.48
0.500	2.52	1.500	50.21	2.500	3.95	3.50	2.48
0.583	2.88	1.583	13.37	2.583	3.56	3.58	2.35
0.667	2.88	1.667	13.37	2.667	3.56	3.67	2.35
0.750	3.38	1.750	8.29	2.750	3.25	3.75	2.23
0.833	3.38	1.833	8.29	2.833	3.25	3.83	2.23
0.917	4.17	1.917	6.30	2.917	3.01	3.92	2.14
1.000	4.18	2.000	6.29	3.000	3.01	4.00	2.14

Unit Hyd Qpeak (cms)= 0.437

PEAK FLOW (cms)= 0.004 (i)
TIME TO PEAK (hrs)= 1.667
RUNOFF VOLUME (mm)= 1.096
TOTAL RAINFALL (mm)= 24.996
RUNOFF COEFFICIENT = 0.044

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (0201)	Area (ha)= 12.44	Dir. Conn.(%)= 36.00
ID= 1 DT= 5.0 min	Total Imp(%)= 71.00		

Surface Area	(ha)=	IMPERVIOUS	PERVIOUS (i)
Dep. Storage	(mm)=	8.83	3.61
Average Slope	(%)=	1.00	5.00
Length	(m)=	1.00	2.00
Mannings n	=	287.98	40.00
		0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.07	1.083	5.70	2.083	5.19	3.08	2.80
0.167	2.07	1.167	5.70	2.167	5.19	3.17	2.80
0.250	2.27	1.250	10.78	2.250	4.47	3.25	2.62
0.333	2.27	1.333	10.78	2.333	4.47	3.33	2.62
0.417	2.52	1.417	50.21	2.417	3.95	3.42	2.48
0.500	2.52	1.500	50.21	2.500	3.95	3.50	2.48
0.583	2.88	1.583	13.37	2.583	3.56	3.58	2.35
0.667	2.88	1.667	13.37	2.667	3.56	3.67	2.35
0.750	3.38	1.750	8.29	2.750	3.25	3.75	2.23
0.833	3.38	1.833	8.29	2.833	3.25	3.83	2.23
0.917	4.17	1.917	6.30	2.917	3.01	3.92	2.14
1.000	4.18	2.000	6.29	3.000	3.01	4.00	2.14

Max.Eff.Inten.(mm/hr)=	50.21	11.56
over (min)=	5.00	25.00
Storage Coeff. (min)=	6.35 (ii)	23.08 (ii)
Unit Hyd. Tpeak (min)=	5.00	25.00
Unit Hyd. peak (cms)=	0.19	0.05

PEAK FLOW (cms)=	0.52	0.06	*TOTALS*
TIME TO PEAK (hrs)=	1.50	1.83	0.536 (iii)
RUNOFF VOLUME (mm)=	24.00	4.57	11.56
TOTAL RAINFALL (mm)=	25.00	25.00	25.00
RUNOFF COEFFICIENT =	0.96	0.18	0.46

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 56.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0501)	OVERFLOW IS OFF
IN= 2---> OUT= 1	
DT= 5.0 min	

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	0.3400	0.4900
0.0000	0.0300	0.3500	0.5400
0.0100	0.0700	0.3700	0.5900
0.0200	0.1000	0.3800	0.6400
0.0200	0.1800	0.4100	0.7400
0.0200	0.2200	0.4200	0.7900
0.0300	0.2600	0.4400	0.8400
0.0800	0.3500	3.9400	0.9500
0.1800	0.4000	6.9400	1.0100
0.3200	0.4400	10.7200	1.0600

INFLOW : ID= 2 (0201)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
OUTFLOW: ID= 1 (0501)	12.440	0.536	1.50	11.56
		0.020	2.50	9.07

PEAK FLOW REDUCTION [Qout/Qin](%)= 3.73
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 0.1241

CALIB
STANDHYD (0203)
ID= 1 DT= 5.0 min

Area (ha)= 0.43
Total Imp(%)= 47.00

Dir. Conn.(%)= 33.00

Surface Area	(ha)=	0.20	PERVIOUS (i)	0.23
Dep. Storage	(mm)=	1.00		5.00
Average slope	(%)=	1.00		2.00
Length	(m)=	53.54		40.00
Mannings n	=	0.013		0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.07	1.083	5.70	2.083	5.19	3.08	2.80
0.167	2.07	1.167	5.70	2.167	5.19	3.17	2.80
0.250	2.27	1.250	10.78	2.250	4.47	3.25	2.62
0.333	2.27	1.333	10.78	2.333	4.47	3.33	2.62
0.417	2.52	1.417	50.21	2.417	3.95	3.42	2.48
0.500	2.52	1.500	50.21	2.500	3.95	3.50	2.48
0.583	2.88	1.583	13.37	2.583	3.56	3.58	2.35
0.667	2.88	1.667	13.37	2.667	3.56	3.67	2.35
0.750	3.38	1.750	8.29	2.750	3.25	3.75	2.23
0.833	3.38	1.833	8.29	2.833	3.25	3.83	2.23
0.917	4.17	1.917	6.30	2.917	3.01	3.92	2.14
1.000	4.18	2.000	6.29	3.000	3.01	4.00	2.14
Max.Eff.Inten.(mm/hr)=		50.21	1.92				
over (min)		5.00	40.00				
Storage Coeff. (min)=		2.31 (ii)	36.61 (ii)				
Unit Hyd. Tpeak (min)=		5.00	40.00				
Unit Hyd. peak (cms)=		0.30	0.03				
PEAK FLOW (cms)=		0.02	0.00		0.020 (iii)		
TIME TO PEAK (hrs)=		1.50	2.25		1.50		
RUNOFF VOLUME (mm)=		24.00	1.92		9.16		
TOTAL RAINFALL (mm)=		25.00	25.00		25.00		
RUNOFF COEFFICIENT		0.96	0.08		0.37		

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0402)
1 + 2 = 3

AREA (ha)
QPEAK (cms)
TPEAK (hrs)
R.V. (mm)

ID1= 1 (0202): 1.60 0.004 1.67 1.10
+ ID2= 2 (0203): 0.43 0.020 1.50 9.16
=====

ID = 3 (0402): 2.03 0.022 1.50 2.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0402)
3 + 2 = 1

AREA (ha)
QPEAK (cms)
TPEAK (hrs)
R.V. (mm)

ID1= 3 (0402): 2.03 0.022 1.50 2.80
+ ID2= 2 (0501): 12.44 0.020 2.50 9.07
=====

ID = 1 (0402): 14.47 0.024 2.50 8.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

junction Command(0901)

AREA QPEAK TPEAK R.V.

INFLOW : ID= 1(0402) 14.47 0.02 2.50 8.19
OUTFLOW: ID= 2(0901) 14.47 0.02 2.50 8.19

Max.Eff.Inten.(mm/hr)= 53.00 112.33
over (min)= 5.00 15.00
Storage Coeff. (min)= 6.21 (ii) 12.95 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.19 0.08

PEAK FLOW (cms)= 0.66 1.11 *TOTALS*
TIME TO PEAK (hrs)= 10.00 10.00 1.765 (iii)
RUNOFF VOLUME (mm)= 211.00 177.30 189.43
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 1.00 0.84 0.89

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2----> OUT= 1					
DT= 5.0 min					
		OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	0.3400	0.4900
		0.0000	0.0300	0.3500	0.5400
		0.0100	0.0700	0.3700	0.5900
		0.0200	0.1000	0.3800	0.6400
		0.0200	0.1800	0.4100	0.7400
		0.0200	0.2200	0.4200	0.7900
		0.0300	0.2600	0.4400	0.8400
		0.0800	0.3500	3.9400	0.9500
		0.1800	0.4000	6.9400	1.0100
		0.3200	0.4400	10.7200	1.0600
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0201)		12.440	1.765	10.00	189.43
OUTFLOW: ID= 1 (0501)		12.440	1.718	10.00	186.94
PEAK FLOW REDUCTION		[Qout/Qin](%)= 97.33			
TIME SHIFT OF PEAK FLOW		(min)= 0.00			
MAXIMUM STORAGE USED		(ha.m.)= 0.8809			

CALIB STANDHYD (0203) ID= 1 DT= 5.0 min									
		Area (ha)= 0.43					Dir. Conn.(%)= 33.00		
		Total Imp(%)= 47.00							
		IMPERVIOUS					PERVIOUS (i)		
Surface Area (ha)= 0.20									
Dep. Storage (mm)= 1.00									
Average Slope (%)= 1.00									
Length (m)= 53.54									
Mannings n = 0.013									

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

		----- TRANSFORMED HYETOGRAPH -----									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00	0.083	6.00	3.083	13.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00	0.167	6.00	3.167	13.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00	0.250	6.00	3.250	13.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00	0.333	6.00	3.333	13.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00	0.417	6.00	3.417	13.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00	0.500	6.00	3.500	13.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00	0.583	6.00	3.583	13.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00	0.667	6.00	3.667	13.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00	0.750	6.00	3.750	13.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00	0.833	6.00	3.833	13.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00	0.917	6.00	3.917	13.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00	1.000	6.00	4.000	13.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00	1.083	4.00	4.083	17.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00	1.167	4.00	4.167	17.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00	1.250	4.00	4.250	17.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00	1.333	4.00	4.333	17.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00	1.417	4.00	4.417	17.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00	1.500	4.00	4.500	17.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00	1.583	4.00	4.583	17.00

1.667 4.00 4.667 17.00 7.667 13.00 10.67 38.00
1.750 4.00 4.750 17.00 7.750 13.00 10.75 38.00
1.833 4.00 4.833 17.00 7.833 13.00 10.83 38.00
1.917 4.00 4.917 17.00 7.917 13.00 10.92 38.00
2.000 4.00 5.000 17.00 8.000 13.00 11.00 38.00
2.083 6.00 5.083 13.00 8.083 13.00 11.08 13.00
2.167 6.00 5.167 13.00 8.167 13.00 11.17 13.00
2.250 6.00 5.250 13.00 8.250 13.00 11.25 13.00
2.333 6.00 5.333 13.00 8.333 13.00 11.33 13.00
2.417 6.00 5.417 13.00 8.417 13.00 11.42 13.00
2.500 6.00 5.500 13.00 8.500 13.00 11.50 13.00
2.583 6.00 5.583 13.00 8.583 13.00 11.58 13.00
2.667 6.00 5.667 13.00 8.667 13.00 11.67 13.00
2.750 6.00 5.750 13.00 8.750 13.00 11.75 13.00
2.833 6.00 5.833 13.00 8.833 13.00 11.83 13.00
2.917 6.00 5.917 13.00 8.917 13.00 11.92 13.00
3.000 6.00 6.000 13.00 9.000 13.00 12.00 13.00

Max.Eff.Inten.(mm/hr)= 53.00 57.74
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.26 (ii) 11.06 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.30 0.09

PEAK FLOW (cms)= 0.02 0.04 *TOTALS*
TIME TO PEAK (hrs)= 9.58 10.00 0.057 (iii)
RUNOFF VOLUME (mm)= 211.00 145.09 10.00
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 1.00 0.68 0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0402) 1 + 2 = 3									
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)			R.V. (mm)		
ID1= 1 (0202):		1.60	0.190	10.00			132.33		
+ ID2= 2 (0203):		0.43	0.057	10.00			166.83		
ID = 3 (0402):		2.03	0.247	10.00			139.64		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0402) 3 + 2 = 1									
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)			R.V. (mm)		
ID1= 3 (0402):		2.03	0.247	10.00			139.64		
+ ID2= 2 (0501):		12.44	1.718	10.00			186.94		
ID = 1 (0402):		14.47	1.965	10.00			180.30		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Junction Command(0901)

		AREA (ha)	QPEAK (cms)	TPEAK (hrs)			R.V. (mm)		
INFLOW : ID= 1(0402)		14.47	1.96	10.00			180.30		
OUTFLOW: ID= 2(0901)		14.47	1.96	10.00			180.30		

FINISH

=====

V V I SSSSS U U A L (v 6.2.2015)

V V I SS U U A A L

V V I SS U U AAAAA L

V V I SS U U A A L

V V I SSSSS UUUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM

O O T T H H Y Y MM MM O O

O O T T H H Y Y M M O O

000 T T H H Y Y M M 000

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voim.dat

Output filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\c476bd59-

Summary filename: C:\Users\lcarretas\AppData\Local\Civica\vh5\13981d5a-de9d-475d-ae56-06a5922f796b\c476bd59-

DATE: 10/02/2024 TIME: 02:34:10

USER:

COMMENTS: _____

** SIMULATION : Hazel **

READ STORM	Filename: C:\Users\lcarretas\AppData\Local\Temp\fd8de6aa-1645-4195-9cee-1a385af14e31\9e68e583
Ptotal=212.00 mm	Comments: Hazel

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	6.00	3.00	13.00	6.00	23.00	9.00	53.00
1.00	4.00	4.00	17.00	7.00	13.00	10.00	38.00
2.00	6.00	5.00	13.00	8.00	13.00	11.00	13.00

CALIB			
NASHYD (0202)	Area (ha)=	1.60	Curve Number (CN)= 70.0
ID= 1 DT= 5.0 min	Ia (mm)=	7.65	# of Linear Res.(N)= 3.00
	U.H. Tp(hrs)=	0.14	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00

1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Unit Hyd Qpeak (cms)= 0.437

PEAK FLOW (cms)= 0.190 (i)

TIME TO PEAK (hrs)= 10.000

RUNOFF VOLUME (mm)= 132.328

TOTAL RAINFALL (mm)= 212.000

RUNOFF COEFFICIENT = 0.624

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
STANDHYD (0203)	Area (ha)=	0.43	
ID= 1 DT= 5.0 min	Total Imp(%)=	47.00	Dir. Conn.(%)= 33.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.20	0.23
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	53.54	40.00
Mannings n	= 0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00

Max.Eff.Inten.(mm/hr)= 53.00 57.74
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.26 (ii) 11.06 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.30 0.09

PEAK FLOW (cms)= 0.02 0.04
TIME TO PEAK (hrs)= 9.58 10.00 10.00
RUNOFF VOLUME (mm)= 211.00 145.09 166.83
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 1.00 0.68 0.79

TOTALS
0.057 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0201)
ID= 1 DT= 5.0 min
Area (ha)= 12.44
Total Imp(%)= 71.00 Dir. Conn.(%)= 36.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 8.83 3.61
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 287.98 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		TIME		RAIN		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.00	3.083	13.00	6.083	23.00	9.08	53.00				
0.167	6.00	3.167	13.00	6.167	23.00	9.17	53.00				
0.250	6.00	3.250	13.00	6.250	23.00	9.25	53.00				
0.333	6.00	3.333	13.00	6.333	23.00	9.33	53.00				
0.417	6.00	3.417	13.00	6.417	23.00	9.42	53.00				
0.500	6.00	3.500	13.00	6.500	23.00	9.50	53.00				
0.583	6.00	3.583	13.00	6.583	23.00	9.58	53.00				
0.667	6.00	3.667	13.00	6.667	23.00	9.67	53.00				
0.750	6.00	3.750	13.00	6.750	23.00	9.75	53.00				
0.833	6.00	3.833	13.00	6.833	23.00	9.83	53.00				
0.917	6.00	3.917	13.00	6.917	23.00	9.92	53.00				
1.000	6.00	4.000	13.00	7.000	23.00	10.00	53.00				
1.083	4.00	4.083	17.00	7.083	13.00	10.08	38.00				
1.167	4.00	4.167	17.00	7.167	13.00	10.17	38.00				
1.250	4.00	4.250	17.00	7.250	13.00	10.25	38.00				
1.333	4.00	4.333	17.00	7.333	13.00	10.33	38.00				
1.417	4.00	4.417	17.00	7.417	13.00	10.42	38.00				
1.500	4.00	4.500	17.00	7.500	13.00	10.50	38.00				
1.583	4.00	4.583	17.00	7.583	13.00	10.58	38.00				
1.667	4.00	4.667	17.00	7.667	13.00	10.67	38.00				
1.750	4.00	4.750	17.00	7.750	13.00	10.75	38.00				
1.833	4.00	4.833	17.00	7.833	13.00	10.83	38.00				
1.917	4.00	4.917	17.00	7.917	13.00	10.92	38.00				
2.000	4.00	5.000	17.00	8.000	13.00	11.00	38.00				
2.083	6.00	5.083	13.00	8.083	13.00	11.08	13.00				
2.167	6.00	5.167	13.00	8.167	13.00	11.17	13.00				
2.250	6.00	5.250	13.00	8.250	13.00	11.25	13.00				
2.333	6.00	5.333	13.00	8.333	13.00	11.33	13.00				
2.417	6.00	5.417	13.00	8.417	13.00	11.42	13.00				
2.500	6.00	5.500	13.00	8.500	13.00	11.50	13.00				
2.583	6.00	5.583	13.00	8.583	13.00	11.58	13.00				
2.667	6.00	5.667	13.00	8.667	13.00	11.67	13.00				
2.750	6.00	5.750	13.00	8.750	13.00	11.75	13.00				
2.833	6.00	5.833	13.00	8.833	13.00	11.83	13.00				
2.917	6.00	5.917	13.00	8.917	13.00	11.92	13.00				
3.000	6.00	6.000	13.00	9.000	13.00	12.00	13.00				

Max.Eff.Inten.(mm/hr)= 53.00 112.33
over (min)= 5.00 15.00
Storage Coeff. (min)= 6.21 (ii) 12.95 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.19 0.08

PEAK FLOW (cms)= 0.66 1.11

TOTALS
1.765 (iii)

TIME TO PEAK (hrs)= 10.00 10.00 10.00
RUNOFF VOLUME (mm)= 211.00 177.30 189.43
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 1.00 0.84 0.89

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0402)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0201): 12.44 1.765 10.00 189.43
+ ID2= 2 (0202): 1.60 0.190 10.00 132.33
=====

ID = 3 (0402): 14.04 1.955 10.00 182.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0402)
3 + 2 = 1
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 3 (0402): 14.04 1.955 10.00 182.93
+ ID2= 2 (0203): 0.43 0.057 10.00 166.83
=====

ID = 1 (0402): 14.47 2.012 10.00 182.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Junction Command(0901)

AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
INFLOW : ID= 1(0402) 14.47 2.01 10.00 182.45
OUTFLOW: ID= 2(0901) 14.47 2.01 10.00 182.45

FINISH

Appendix D: Stage-Storage-Discharge Tables

Maple Bridge Subdivision, Phase 2
SWM Pond Volume Table - Dead Storage

Designed: LJC
 Checked: JG
 Date: 10/21/2024

Wet Pond Characteristics:

Top Elevation: 269.30 m
 Bottom Elevation: 265.00 m
 Permanent Pool: 267.00 m
 Stage: 0.1 m

Stormwater Management Pond - Dead Storage											
Pond Geometry							Pond Volume (m ³)				
Elevation (m)	Depth (m)	Wet Cell Area (m ²)	Forebay Area (m ²)	Avg. Wet Cell Area (m ²)	Avg. Forebay Area (m ²)	Total Area	Wet Cell Volume	Forebay Volume	Accum. Wet Cell	Accum. Forebay	Accum. Dead
265.00	0.00	1223	0	1223	0	1223	0.00	0.00	0.00	0.00	0.00
265.10	0.10	1262	0	1242	0	1262	124.24	0.00	124.24	0.00	124.24
265.20	0.20	1300	0	1281	0	1300	128.11	0.00	252.34	0.00	252.34
265.30	0.30	1339	0	1320	0	1339	131.98	0.00	384.32	0.00	384.32
265.40	0.40	1378	0	1359	0	1378	135.85	0.00	520.17	0.00	520.17
265.50	0.50	1417	0	1397	0	1417	139.72	0.00	659.89	0.00	659.89
265.60	0.60	1455	0	1436	0	1455	143.59	0.00	803.49	0.00	803.49
265.70	0.70	1494	0	1475	0	1494	147.46	0.00	950.95	0.00	950.95
265.80	0.80	1533	0	1513	0	1533	151.34	0.00	1102.29	0.00	1102.29
265.90	0.90	1571	0	1552	0	1571	155.21	0.00	1257.49	0.00	1257.49
266.00	1.00	1610	428	1591	428	2038	159.08	0.00	1416.57	0.00	1416.57
266.10	1.10	1649	461	1630	444	2109	162.95	44.43	1579.52	44.43	1623.95
266.20	1.20	1688	493	1668	477	2181	166.82	47.68	1746.34	92.10	1838.44
266.30	1.30	1726	526	1707	509	2252	170.69	50.93	1917.04	143.03	2060.06
266.40	1.40	1765	558	1746	542	2323	174.56	54.18	2091.60	197.20	2288.80
266.50	1.50	1847	619	1806	588	2466	180.62	58.84	2272.22	256.04	2528.26
266.60	1.60	1930	680	1889	649	2609	188.85	64.93	2461.07	320.97	2782.03
266.70	1.70	2012	741	1971	710	2753	197.08	71.01	2658.15	391.98	3050.13
266.80	1.80	2094	801	2053	771	2896	205.32	77.09	2863.47	469.07	3332.53
266.90	1.90	2177	862	2136	832	3039	213.55	83.18	3077.02	552.24	3629.26
267.00	2.00	2259	923	2218	893	3182	221.78	89.26	3298.80	641.50	3940.30

Maple Bridge Subdivision, Phase 2
SWM Pond Volume Table - Total Storage

Designed: LJC
 Checked: JG
 Date: 10/21/2024

Wet Pond Characteristics:

Top Elevation: 269.30 m
 Bottom Elev: 265.00 m
 Permanent Pool: 267.00 m
 Stage 0.1 m

Stormwater Management Pond							
Pond Geometry				Pond Volume (m ³)			
Elevation (m)	Depth (m)	Area (m ²)	Avg. Area (m)	Dead	Accum. Dead	Live	Accum. Live
265.00	0.00	1223	1223.00	0.00	0.00	0.00	0.00
265.10	0.10	1262	1242.36	124.24	124.24	0.00	0.00
265.20	0.20	1300	1281.07	128.11	252.34	0.00	0.00
265.30	0.30	1339	1319.79	131.98	384.32	0.00	0.00
265.40	0.40	1378	1358.50	135.85	520.17	0.00	0.00
265.50	0.50	1417	1397.21	139.72	659.89	0.00	0.00
265.60	0.60	1455	1435.93	143.59	803.49	0.00	0.00
265.70	0.70	1494	1474.64	147.46	950.95	0.00	0.00
265.80	0.80	1533	1513.36	151.34	1102.29	0.00	0.00
265.90	0.90	1571	1552.07	155.21	1257.49	0.00	0.00
266.00	1.00	2038	1804.79	180.48	1416.57	0.00	0.00
266.10	1.10	2109	2073.75	207.38	1623.95	0.00	0.00
266.20	1.20	2181	2144.96	214.50	1838.44	0.00	0.00
266.30	1.30	2252	2216.18	221.62	2060.06	0.00	0.00
266.40	1.40	2323	2287.39	228.74	2288.80	0.00	0.00
266.50	1.50	2466	2394.58	239.46	2528.26	0.00	0.00
266.60	1.60	2609	2537.75	253.78	2782.03	0.00	0.00
266.70	1.70	2753	2680.92	268.09	3050.13	0.00	0.00
266.80	1.80	2896	2824.08	282.41	3332.53	0.00	0.00
266.90	1.90	3039	2967.25	296.73	3629.26	0.00	0.00
267.00	2.00	3182	3110.42	311.04	3940.30	0.00	0.00
267.10	2.10	3350	3266.08	311.04	3940.30	326.61	326.61
267.20	2.20	3518	3434.25	311.04	3940.30	343.43	670.03
267.30	2.30	3687	3602.42	311.04	3940.30	360.24	1030.28
267.40	2.40	3855	3770.58	311.04	3940.30	377.06	1407.33
267.50	2.50	4023	3938.75	311.04	3940.30	393.88	1801.21
267.60	2.60	4191	4106.92	311.04	3940.30	410.69	2211.90
267.70	2.70	4282	4236.29	311.04	3940.30	423.63	2635.53
267.80	2.80	4372	4326.88	311.04	3940.30	432.69	3068.22
267.90	2.90	4463	4417.47	311.04	3940.30	441.75	3509.96
268.00	3.00	4553	4508.06	311.04	3940.30	450.81	3960.77
268.10	3.10	4644	4598.65	311.04	3940.30	459.86	4420.64
268.20	3.20	4735	4689.24	311.04	3940.30	468.92	4889.56
268.30	3.30	4825	4779.82	311.04	3940.30	477.98	5367.54
268.40	3.40	4916	4870.41	311.04	3940.30	487.04	5854.58
268.50	3.50	5006	4961.00	311.04	3940.30	496.10	6350.68
268.60	3.60	5097	5051.59	311.04	3940.30	505.16	6855.84
268.70	3.70	5187	5142.18	311.04	3940.30	514.22	7370.06
268.80	3.80	5278	5232.76	311.04	3940.30	523.28	7893.34
268.90	3.90	5369	5323.35	311.04	3940.30	532.34	8425.67
269.00	4.00	5459	5413.94	311.04	3940.30	541.39	8967.06
269.10	4.10	5550	5504.53	311.04	3940.30	550.45	9517.52
269.20	4.20	5640	5595.12	311.04	3940.30	559.51	10077.03
269.30	4.30	5731	5685.71	311.04	3940.30	568.57	10645.60

Maple Bridge Subdivision, Phase 2
SWM Pond Discharge Table

Designed: LJC
 Checked: JG
 Date: 10/21/2024

Pond Discharge Table:

Orifice #1:		Orifice #2:		Outlet Pipe		Overflow Weir:	
Diameter: 120		Diameter: 370	mm	Diameter: 600		Bottom Length: 27	m
Area: 0.011		Area: 0.108	m ²	Area: 0.283		Sill Elevation: 268.90	m
C: 0.63		C: 0.63		C: 0.80		D/S Weir Length: 15.4	m
Invert: 267.00		Invert: 267.00	m	Invert: 266.85		Side Slopes (H:V) 5	:1
						Weir Depth: 0.40	m

Elevation (m)	Orifice #1		Orifice #2		Outlet Pipe		Overflow Weir		Hydraulic Control	Discharge (m ³ /s)
	Head (m)	Discharge (m)	Head (m)	Discharge (m)	Head (m)	Discharge (m)	Head (m)	Discharge (m)		
267.00	0.000	0.000	0.000	0.000	0.150	0.026	0	0.000	Orifice	0.000
267.10	0.040	0.005	0.000	0.000	0.250	0.072	0	0.000	Orifice	0.005
267.20	0.140	0.012	0.015	0.000	0.350	0.136	0	0.000	Orifice	0.012
267.30	0.240	0.015	0.115	0.000	0.450	0.212	0	0.000	Orifice	0.015
267.40	0.340	0.018	0.215	0.000	0.550	0.293	0	0.000	Orifice	0.018
267.50	0.440	0.021	0.315	0.000	0.650	0.808	0	0.000	Orifice	0.021
267.60	0.540	0.023	0.415	0.000	0.750	0.868	0	0.000	Orifice	0.023
267.70	0.640	0.025	0.515	0.000	0.850	0.924	0	0.000	Orifice	0.025
267.80	0.740	0.027	0.615	0.000	0.950	0.977	0	0.000	Orifice	0.027
267.90	0.840	0.029	0.715	0.049	1.050	1.027	0	0.000	Orifice	0.078
268.00	0.940	0.031	0.815	0.153	1.150	1.074	0	0.000	Orifice	0.184
268.10	1.040	0.032	0.915	0.287	1.250	1.120	0	0.000	Orifice	0.319
268.20	1.140	0.034	1.015	0.302	1.350	1.164	0	0.000	Orifice	0.336
268.30	1.240	0.035	1.115	0.317	1.450	1.206	0	0.000	Orifice	0.352
268.40	1.340	0.037	1.215	0.331	1.550	1.247	0	0.000	Orifice	0.367
268.50	1.440	0.038	1.315	0.344	1.650	1.287	0	0.000	Orifice	0.382
268.60	1.540	0.039	1.415	0.357	1.750	1.325	0	0.000	Orifice	0.396
268.70	1.640	0.040	1.515	0.369	1.850	1.363	0	0.000	Orifice	0.410
268.80	1.740	0.042	1.615	0.381	1.950	1.399	0	0.000	Orifice	0.423
268.90	1.840	0.043	1.715	0.393	2.050	1.435	9.1E-13	0.000	Orifice	0.436
269.00	1.940	0.044	1.815	0.404	2.150	1.469	0.1	1.212	Orifice	1.660
269.10	2.040	0.045	1.915	0.415	2.250	1.503	0.2	3.475	Orifice	3.935
269.20	2.140	0.046	2.015	0.426	2.350	1.536	0.3	6.470	Orifice	6.942
269.30	2.240	0.047	2.115	0.436	2.450	1.568	0.4	10.234	Orifice	10.717

Notes:

Orifice Equation is: $Q = C \times A \times (2gH)^{0.5}$

Where:

- Q = flow rate (cms)
- C = constant
- A = area of opening(sq. m)
- H = net head on the orifice
- g = Acceleration due to gravity

Maple Bridge Subdivision, Phase 2
SWM Pond - Partially Full Orifice - #1

Designed: LJC
Checked: JG
Date: 10/21/2024

Partially Full Orifice Equation

Orifice Equation
 $Q = C \times A \times (2gH)^{0.5}$
Q = flow rate (m³)
C = constant
A = area of opening (m²)
H = net head on the orifice
g = Acceleration due to gravity

Orifice Constant (C): **0.63**
Depth Increment (m): **0.10**
Minimum Elevation (Invert of Control) (m): **267.00**
Maximum Elevation (Top of SWMF) (m): **269.30**

Control Features

Orifice Diameter (m): **0.120**
Mannings Value: **0.013**
Invert Elevation: **267.00**
Inlet Type: **2**
1 = Projecting
2 = Flush

Inlet Control Equation (Smith) Projecting Inlet

MMF Model $y = (a*b + c*x^d) / (b + x^d)$

Coefficient Data

a = 0.0168
b = 1.77
c = 3.06
d = 2.27

Inlet Control Equation (Smith) Flush Inlet

MMF Model $y = (a*b + c*x^d) / (b + x^d)$

Coefficient Data

a = -1.08E-03
b = 2.30E+00
c = 3.95
d = 2.08

Water Elevation	Depth from Min. Elevation	Pipe Partially Full Inlet Control Flow
m	m	m ³ /s
267.00	0.00	0.000
267.10	0.10	0.005
267.20	0.20	0.012
267.30	0.30	0.015
267.40	0.40	0.018
267.50	0.50	0.021
267.60	0.60	0.023
267.70	0.70	0.025
267.80	0.80	0.027
267.90	0.90	0.029
268.00	1.00	0.031
268.10	1.10	0.032
268.20	1.20	0.034
268.30	1.30	0.035
268.40	1.40	0.037
268.50	1.50	0.038
268.60	1.60	0.039
268.70	1.70	0.040
268.80	1.80	0.042
268.90	1.90	0.043
269.00	2.00	0.044
269.10	2.10	0.045
269.20	2.20	0.046
269.30	2.30	0.047

Maple Bridge Subdivision, Phase 2
SWM Pond - Partially Full Orifice - Outlet Pipe

Designed: LJC
 Checked: JG
 Date: 10/21/2024

Partially Full Orifice Equation

Orifice Equation
 $Q = C \times A \times (2gH)^{0.5}$
 Q = flow rate (m³)
 C = constant
 A = area of opening (m²)
 H = net head on the orifice
 g = Acceleration due to gravity

Orifice Constant (C): **0.80**
 Depth Increment (m): **0.10**
 Minimum Elevation (Invert of Control) (m): **266.85**
 Maximum Elevation (Top of SWMF) (m): **269.30**

Control Features

Orifice Diameter (m): **0.600**
 Mannings Value: **0.013**
 Invert Elevation: **266.85**
 Inlet Type: **2**
 1 = Projecting
 2 = Flush

Inlet Control Equation (Smith) Projecting Inlet

MMF Model $y = (a*b + c*x^d) / (b + x^d)$

Coefficient Data
 a = 0.0168
 b = 1.77
 c = 3.06
 d = 2.27

Inlet Control Equation (Smith) Flush Inlet

MMF Model $y = (a*b + c*x^d) / (b + x^d)$

Coefficient Data
 a = -1.08E-03
 b = 2.30E+00
 c = 3.95
 d = 2.08

Water Elevation	Depth from Min. Elevation	Pipe Partially Full Inlet Control Flow
m	m	m ³ /s
266.85	0.00	0.000
266.90	0.05	0.002
267.00	0.15	0.026
267.10	0.25	0.072
267.20	0.35	0.136
267.30	0.45	0.212
267.40	0.55	0.293
267.50	0.65	0.374
267.60	0.75	0.450
267.70	0.85	0.521
267.80	0.95	0.808
267.90	1.05	0.868
268.00	1.15	0.924
268.10	1.25	0.977
268.20	1.35	1.027
268.30	1.45	1.074
268.40	1.55	1.120
268.50	1.65	1.164
268.60	1.75	1.206
268.70	1.85	1.247
268.80	1.95	1.287
268.90	2.05	1.325
269.00	2.15	1.363
269.10	2.25	1.399
269.20	2.35	1.435
269.30	2.45	1.469

Maple Bridge Subdivision, Phase 2
Discharge-Volume

Designed:	<u>LJC</u>
Checked:	<u>JG</u>
Date:	<u>10/21/2024</u>

<u>Elevation</u>	<u>Discharge</u>	<u>Volume</u>
267.00	0.00	0.00
267.10	0.00	326.61
267.20	0.01	670.03
267.30	0.02	1030.28
267.40	0.02	1407.33
267.50	0.02	1801.21
267.60	0.02	2211.90
267.70	0.03	2635.53
267.80	0.03	3068.22
267.90	0.08	3509.96
268.00	0.18	3960.77
268.10	0.32	4420.64
268.20	0.34	4889.56
268.30	0.35	5367.54
268.40	0.37	5854.58
268.50	0.38	6350.68
268.60	0.40	6855.84
268.70	0.41	7370.06
268.80	0.42	7893.34
268.90	0.44	8425.67
269.00	1.66	8967.06
269.10	3.94	9517.52
269.20	6.94	10077.03
269.30	10.72	10645.60

Maple Bridge Subdivision, Phase 2
Stage-Storage-Discharge

Designed: LJC
 Checked: JG
 Date: 10/21/2024

Stormwater Management Pond							
Pond Geometry				Pond Volume (m ³)			Discharge
Elevation (m)	Depth (m)	Area (m ²)	Avg. Area (m)	Dead	Live	Total	(m ³ /s)
265.00	0.00	1223.0	1223.0	0	0	0.0	0.000
265.10	0.10	1261.7	1242.4	124	0	124.2	0.000
265.20	0.20	1300.4	1281.1	252	0	252.3	0.000
265.30	0.30	1339.1	1319.8	384	0	384.3	0.000
265.40	0.40	1377.9	1358.5	520	0	520.2	0.000
265.50	0.50	1416.6	1397.2	660	0	659.9	0.000
265.60	0.60	1455.3	1435.9	803	0	803.5	0.000
265.70	0.70	1494.0	1474.6	951	0	951.0	0.000
265.80	0.80	1532.7	1513.4	1102	0	1102.3	0.000
265.90	0.90	1571.4	1552.1	1257	0	1257.5	0.000
266.00	1.00	2038.1	1804.8	1417	0	1416.6	0.000
266.10	1.10	2109.4	2073.8	1624	0	1623.9	0.000
266.20	1.20	2180.6	2145.0	1838	0	1838.4	0.000
266.30	1.30	2251.8	2216.2	2060	0	2060.1	0.000
266.40	1.40	2323.0	2287.4	2289	0	2288.8	0.000
266.50	1.50	2466.2	2394.6	2528	0	2528.3	0.000
266.60	1.60	2609.3	2537.8	2782	0	2782.0	0.000
266.70	1.70	2752.5	2680.9	3050	0	3050.1	0.000
266.80	1.80	2895.7	2824.1	3333	0	3332.5	0.000
266.90	1.90	3038.8	2967.3	3629	0	3629.3	0.000
267.00	2.00	3182.0	3110.4	3940	0	3940.3	0.000
267.10	2.10	3350.2	3266.1	3940	327	4266.9	0.005
267.20	2.20	3518.3	3434.3	3940	670	4610.3	0.012
267.30	2.30	3686.5	3602.4	3940	1030	4970.6	0.015
267.40	2.40	3854.7	3770.6	3940	1407	5347.6	0.018
267.50	2.50	4022.8	3938.8	3940	1801	5741.5	0.021
267.60	2.60	4191.0	4106.9	3940	2212	6152.2	0.023
267.70	2.70	4281.6	4236.3	3940	2636	6575.8	0.025
267.80	2.80	4372.2	4326.9	3940	3068	7008.5	0.027
267.90	2.90	4462.8	4417.5	3940	3510	7450.3	0.078
268.00	3.00	4553.4	4508.1	3940	3961	7901.1	0.184
268.10	3.10	4643.9	4598.6	3940	4421	8360.9	0.319
268.20	3.20	4734.5	4689.2	3940	4890	8829.9	0.336
268.30	3.30	4825.1	4779.8	3940	5368	9307.8	0.352
268.40	3.40	4915.7	4870.4	3940	5855	9794.9	0.367
268.50	3.50	5006.3	4961.0	3940	6351	10291.0	0.382
268.60	3.60	5096.9	5051.6	3940	6856	10796.1	0.396
268.70	3.70	5187.5	5142.2	3940	7370	11310.4	0.410
268.80	3.80	5278.1	5232.8	3940	7893	11833.6	0.423
268.90	3.90	5368.6	5323.4	3940	8426	12366.0	0.436
269.00	4.00	5459.2	5413.9	3940	8967	12907.4	1.660
269.10	4.10	5549.8	5504.5	3940	9518	13457.8	3.935
269.20	4.20	5640.4	5595.1	3940	10077	14017.3	6.942
269.30	4.30	5731.0	5685.7	3940	10646	14585.9	10.717

Total Dead Storage = 3,940 m³
 Total Live Storage = 10,646 m³

Maple Bridge Subdivision, Phase 2
Operation Characteristics

Designed: LJC
 Checked: JG
 Date: 10/21/2024

STORM EVENT	VOLUME (m ³)	DISCHARGE (m ³ /s)	WATER ELEVATION (m)
4-hr CHI			
1:2-year	1,846	0.02	267.51
1:5-year	2,712	0.04	267.72
1:10-year	3,177	0.06	267.82
1:25-year	3,681	0.12	267.94
1:50-year	3,913	0.16	267.99
1:100-year	4,134	0.23	268.04
25 mm	1,241	0.02	267.36

12-hr SCS			
1:2-year	2,584	0.03	267.69
1:5-year	3,405	0.08	267.88
1:10-year	3,839	0.15	267.97
1:25-year	4,380	0.31	268.09
1:50-year	4,893	0.34	268.20
1:100-year	5,521	0.36	268.33

24-hr SCS			
1:2-year	2,860	0.04	267.75
1:5-year	3,750	0.13	267.95
1:10-year	4,228	0.26	268.06
1:25-year	4,977	0.34	268.22
1:50-year	5,704	0.36	268.37
1:100-year	6,442	0.38	268.52
Hazel	8,809	1.72	269.00
Hazel (Uncontrolled)	-	1.77	269.00

Maple Bridge Subdivision, Phase 2
FILE NO. 422492

25mm 4 HOUR DRAWDOWN TIME FOR POND - ULTIMATE CONDITIONS

(Using the falling head orifice equation)

$$t = \frac{2 A_p}{C A_o (2g)^{0.5}} (h_1^{0.5} - h_2^{0.5})$$

Value

where t = drawdown time in seconds

A_p = surface area of pond (m^2)

3,777 m^2

C = discharge coefficient (typically 0.63)

0.63

A_o = cross-sectional area of orifice (m^2)

0.01131 m^2 for

120 mm dia

g = gravitational acceleration constant ($9.81 m/s^2$)

9.81 m/s^2

h_1 = starting water elevation above the orifice (m)

0.356 m

h_2 = ending water elevation above the orifice (m)

0.000 m

t = 142,772.06 seconds

Q = 0.0188 m^3/s

t = 39.66 hours

Maple Bridge Subdivision, Phase 2
FILE NO. 422492

25mm 4 HOUR DRAWDOWN TIME FOR POND - EXTENDED DETENTION

(Using the falling head orifice equation)

$$t = \frac{2 A_p}{C A_o (2g)^{0.5}} (h_1^{0.5} - h_2^{0.5})$$

Value

where t = drawdown time in seconds

A_p = surface area of pond (m^2)

4,327 m^2

C = discharge coefficient (typically 0.63)

0.63

A_o = cross-sectional area of orifice (m^2)

0.01131 m^2 for

120 mm dia

g = gravitational acceleration constant ($9.81 m/s^2$)

9.81 m/s^2

h_1 = starting water elevation above the orifice (m)

0.800 m

h_2 = ending water elevation above the orifice (m)

0.000 m

t = 245,249.10 seconds

Q = 0.0282 m^3/s

t = 68.12 hours

** SIMULATION:25 mm 4-hr CHI **

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2----> OUT= 1					
DT= 5.0 min					
		OUTFLOW (cms)		STORAGE (ha.m.)	
		0.0000		0.0000	
		0.0000		0.0300	
		0.0100		0.0700	
		0.0200		0.1000	
		0.0200		0.1800	
		0.0200		0.2200	
		0.0300		0.2600	
		0.0800		0.3500	
		0.1800		0.4000	
		0.3200		0.4400	
				10.7200	
		AREA (ha)		QPEAK (cms)	
INFLOW : ID= 2 (0201)		12.440		0.536	
OUTFLOW: ID= 1 (0501)		12.440		0.020	
				TPEAK (hrs)	
				1.50	
				2.50	
				R.V. (mm)	
				11.56	
				9.07	
		PEAK FLOW REDUCTION		[Qout/Qin](%)= 3.73	
		TIME SHIFT OF PEAK FLOW		(min)= 60.00	
		MAXIMUM STORAGE USED		(ha.m.)= 0.1241	

** SIMULATION:Run 01 - 2yr 4hr 10min Chicago **

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2---> OUT= 1					
DT= 5.0 min					
		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	0.3400	0.4900
		0.0000	0.0300	0.3500	0.5400
		0.0100	0.0700	0.3700	0.5900
		0.0200	0.1000	0.3800	0.6400
		0.0200	0.1800	0.4100	0.7400
		0.0200	0.2200	0.4200	0.7900
		0.0300	0.2600	0.4400	0.8400
		0.0800	0.3500	3.9400	0.9500
		0.1800	0.4000	6.9400	1.0100
		0.3200	0.4400	10.7200	1.0600
			AREA	QPEAK	TPEAK
			(ha)	(cms)	(hrs)
INFLOW : ID= 2 (0201)		12.440	0.878	1.33	16.66
OUTFLOW: ID= 1 (0501)		12.440	0.020	1.58	14.16
		PEAK FLOW REDUCTION	[Qout/Qin](%)= 2.28		
		TIME SHIFT OF PEAK FLOW	(min)= 15.00		
		MAXIMUM STORAGE USED	(ha.m.)= 0.1846		

** SIMULATION:Run 02 - 5yr 4hr 10min Chicago **

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2---> OUT= 1					
DT= 5.0 min					

		OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	0.3400	0.4900
		0.0000	0.0300	0.3500	0.5400
		0.0100	0.0700	0.3700	0.5900
		0.0200	0.1000	0.3800	0.6400
		0.0200	0.1800	0.4100	0.7400
		0.0200	0.2200	0.4200	0.7900
		0.0300	0.2600	0.4400	0.8400
		0.0800	0.3500	3.9400	0.9500
		0.1800	0.4000	6.9400	1.0100
		0.3200	0.4400	10.7200	1.0600
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0201)		12.440	1.286	1.33	24.13
OUTFLOW: ID= 1 (0501)		12.440	0.036	4.17	21.63
		PEAK FLOW REDUCTION [Qout/Qin](%)= 2.82			
		TIME SHIFT OF PEAK FLOW (min)=170.00			

MAXIMUM STORAGE USED (ha.m.)= 0.2712

** SIMULATION:Run 03 - 10yr 4hr 10min Chicago **

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2----> OUT= 1					
DT= 5.0 min					
		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	0.3400	0.4900
		0.0000	0.0300	0.3500	0.5400
		0.0100	0.0700	0.3700	0.5900
		0.0200	0.1000	0.3800	0.6400
		0.0200	0.1800	0.4100	0.7400
		0.0200	0.2200	0.4200	0.7900
		0.0300	0.2600	0.4400	0.8400
		0.0800	0.3500	3.9400	0.9500
		0.1800	0.4000	6.9400	1.0100
		0.3200	0.4400	10.7200	1.0600
		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0201)		12.440	1.549	1.33	29.24
OUTFLOW: ID= 1 (0501)		12.440	0.062	4.08	26.75
		PEAK FLOW REDUCTION	[Qout/Qin](%)= 4.01		
		TIME SHIFT OF PEAK FLOW	(min)=165.00		
		MAXIMUM STORAGE USED	(ha.m.)= 0.3177		

** SIMULATION:Run 04 - 25yr 4hr 10min Chicago **

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2----> OUT= 1					
DT= 5.0 min					
		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	0.3400	0.4900
		0.0000	0.0300	0.3500	0.5400
		0.0100	0.0700	0.3700	0.5900
		0.0200	0.1000	0.3800	0.6400
		0.0200	0.1800	0.4100	0.7400
		0.0200	0.2200	0.4200	0.7900
		0.0300	0.2600	0.4400	0.8400
		0.0800	0.3500	3.9400	0.9500
		0.1800	0.4000	6.9400	1.0100
		0.3200	0.4400	10.7200	1.0600
		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0201)		12.440	1.900	1.33	36.05
OUTFLOW: ID= 1 (0501)		12.440	0.116	4.00	33.56
		PEAK FLOW REDUCTION	[Qout/Qin](%)= 6.10		
		TIME SHIFT OF PEAK FLOW	(min)=160.00		
		MAXIMUM STORAGE USED	(ha.m.)= 0.3681		

PEAK FLOW REDUCTION [Qout/Qin](%)= 7.50
TIME SHIFT OF PEAK FLOW (min)=125.00
MAXIMUM STORAGE USED (ha.m.)= 0.3913

** SIMULATION:Run 06 - 100yr 4hr 10min Chicago **

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2---> OUT= 1					
DT= 5.0 min					

		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	0.3400	0.4900
		0.0000	0.0300	0.3500	0.5400
		0.0100	0.0700	0.3700	0.5900
		0.0200	0.1000	0.3800	0.6400
		0.0200	0.1800	0.4100	0.7400
		0.0200	0.2200	0.4200	0.7900
		0.0300	0.2600	0.4400	0.8400
		0.0800	0.3500	3.9400	0.9500
		0.1800	0.4000	6.9400	1.0100
		0.3200	0.4400	10.7200	1.0600
		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0201)		12.440	2.717	1.33	46.50
OUTFLOW: ID= 1 (0501)		12.440	0.227	2.83	44.01

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.35
TIME SHIFT OF PEAK FLOW (min)= 90.00
MAXIMUM STORAGE USED (ha.m.)= 0.4134

** SIMULATION:Run 07 - 2yr 12hr 15min SCS Type II (MTO) **

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2--> OUT= 1					
DT= 5.0 min					

		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	0.3400	0.4900
		0.0000	0.0300	0.3500	0.5400
		0.0100	0.0700	0.3700	0.5900
		0.0200	0.1000	0.3800	0.6400
		0.0200	0.1800	0.4100	0.7400
		0.0200	0.2200	0.4200	0.7900
		0.0300	0.2600	0.4400	0.8400
		0.0800	0.3500	3.9400	0.9500
		0.1800	0.4000	6.9400	1.0100
		0.3200	0.4400	10.7200	1.0600
		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0201)		12.440	0.911	6.25	25.52
OUTFLOW: ID= 1 (0501)		12.440	0.030	10.50	23.02

PEAK FLOW REDUCTION [Qout/Qin](%)= 3.25
TIME SHIFT OF PEAK FLOW (min)=255.00
MAXIMUM STORAGE USED (ha.m.)= 0.2584

** SIMULATION:Run 08 - 5yr 12hr 15min SCS Type II (MTO) **

RESERVOIR(0501)		OVERFLOW IS OFF					
IN= 2---> OUT= 1							
DT= 5.0 min							

OUTFLOW		STORAGE		OUTFLOW		STORAGE	
(cms)		(ha.m.)		(cms)		(ha.m.)	
0.0000		0.0000		0.3400		0.4900	
0.0000		0.0300		0.3500		0.5400	
0.0100		0.0700		0.3700		0.5900	
0.0200		0.1000		0.3800		0.6400	
0.0200		0.1800		0.4100		0.7400	
0.0200		0.2200		0.4200		0.7900	
0.0300		0.2600		0.4400		0.8400	
0.0800		0.3500		3.9400		0.9500	
0.1800		0.4000		6.9400		1.0100	
0.3200		0.4400		10.7200		1.0600	
		AREA		QPEAK		TPEAK	
						R.V.	

INFLOW : ID= 2 (0201) (ha) (cms) (hrs) (mm)
12.440 1.418 6.25 36.93
OUTFLOW: ID= 1 (0501) 12.440 0.075 8.42 34.44

PEAK FLOW REDUCTION [Qout/Qin](%)= 5.27
TIME SHIFT OF PEAK FLOW (min)=130.00
MAXIMUM STORAGE USED (ha.m.)= 0.3405

** SIMULATION:Run 09 - 10yr 12hr 15min SCS Type II (MTO) **

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2----> OUT= 1					
DT= 5.0 min					
		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	0.3400	0.4900
		0.0000	0.0300	0.3500	0.5400
		0.0100	0.0700	0.3700	0.5900
		0.0200	0.1000	0.3800	0.6400
		0.0200	0.1800	0.4100	0.7400
		0.0200	0.2200	0.4200	0.7900
		0.0300	0.2600	0.4400	0.8400
		0.0800	0.3500	3.9400	0.9500
		0.1800	0.4000	6.9400	1.0100
		0.3200	0.4400	10.7200	1.0600
		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0201)		12.440	1.733	6.25	44.69
OUTFLOW: ID= 1 (0501)		12.440	0.148	7.33	42.20

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.53
TIME SHIFT OF PEAK FLOW (min)= 65.00
MAXIMUM STORAGE USED (ha.m.)= 0.3839

** SIMULATION:Run 10 - 25yr 12hr 15min SCS Type II (MTO) **

RESERVOIR(0501)		OVERFLOW IS OFF			
IN= 2---> OUT= 1					
DT= 5.0 min					

		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	0.3400	0.4900
		0.0000	0.0300	0.3500	0.5400
		0.0100	0.0700	0.3700	0.5900
		0.0200	0.1000	0.3800	0.6400
		0.0200	0.1800	0.4100	0.7400
		0.0200	0.2200	0.4200	0.7900
		0.0300	0.2600	0.4400	0.8400
		0.0800	0.3500	3.9400	0.9500
		0.1800	0.4000	6.9400	1.0100
		0.3200	0.4400	10.7200	1.0600
		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0201)		12.440	2.158	6.25	54.96
OUTFLOW: ID= 1 (0501)		12.440	0.313	6.92	52.47

PEAK FLOW REDUCTION [Qout/Qin](%)= 14.49
TIME SHIFT OF PEAK FLOW (min)= 40.00
MAXIMUM STORAGE USED (ha.m.)= 0.4380

** SIMULATION:Run 11 - 50yr 12hr 15min SCS Type II (MTO) **

RESERVOIR(0501) IN= 2---> OUT= 1 DT= 5.0 min					OVERFLOW IS OFF				
OUTFLOW (cms)		STORAGE (ha.m.)			OUTFLOW (cms)		STORAGE (ha.m.)		
0.0000		0.0000			0.3400		0.4900		
0.0000		0.0300			0.3500		0.5400		
0.0100		0.0700			0.3700		0.5900		
0.0200		0.1000			0.3800		0.6400		
0.0200		0.1800			0.4100		0.7400		
0.0200		0.2200			0.4200		0.7900		
0.0300		0.2600			0.4400		0.8400		
0.0800		0.3500			3.9400		0.9500		
0.1800		0.4000			6.9400		1.0100		

	0.3200	0.4400		10.7200	1.0600
	AREA	QPEAK		TPEAK	R.V.
	(ha)	(cms)		(hrs)	(mm)
INFLOW : ID= 2 (0201)	12.440	2.482		6.25	62.70
OUTFLOW: ID= 1 (0501)	12.440	0.340		6.92	60.21
PEAK FLOW REDUCTION [Qout/Qin](%)=	13.68				
TIME SHIFT OF PEAK FLOW (min)=	40.00				
MAXIMUM STORAGE USED (ha.m.)=	0.4893				

** SIMULATION:Run 12 - 100yr 12hr 15min SCS Type II (MTO) **

RESERVOIR(0501)	OVERFLOW IS OFF
IN= 2---> OUT= 1	
DT= 5.0 min	
OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000
0.0000	0.0300
0.0100	0.0700
0.0200	0.1000
0.0200	0.1800
0.0200	0.2200
0.0300	0.2600
0.0800	0.3500
0.1800	0.4000
0.3200	0.4400
OUTFLOW (cms)	STORAGE (ha.m.)
0.3400	0.4900
0.3500	0.5400
0.3700	0.5900
0.3800	0.6400
0.4100	0.7400
0.4200	0.7900
0.4400	0.8400
3.9400	0.9500
6.9400	1.0100
10.7200	1.0600
AREA (ha)	QPEAK (cms)
12.440	3.028
12.440	0.355
TPEAK (hrs)	R.V. (mm)
6.25	70.60
6.92	68.10
PEAK FLOW REDUCTION [Qout/Qin](%)=	11.72
TIME SHIFT OF PEAK FLOW (min)=	40.00
MAXIMUM STORAGE USED (ha.m.)=	0.5521

** SIMULATION:Run 13 - 2yr 24hr 15min SCS Type II (MTO) **

RESERVOIR(0501)	OVERFLOW IS OFF
IN= 2---> OUT= 1	
DT= 5.0 min	
OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000
0.0000	0.0300
0.0100	0.0700
0.0200	0.1000
0.0200	0.1800
0.0200	0.2200
0.0300	0.2600
0.0800	0.3500
0.1800	0.4000
0.3200	0.4400
OUTFLOW (cms)	STORAGE (ha.m.)
0.3400	0.4900
0.3500	0.5400
0.3700	0.5900
0.3800	0.6400
0.4100	0.7400
0.4200	0.7900
0.4400	0.8400
3.9400	0.9500
6.9400	1.0100
10.7200	1.0600
AREA (ha)	QPEAK (cms)
12.440	1.139
12.440	0.044
TPEAK (hrs)	R.V. (mm)
12.25	33.37
14.67	30.87
PEAK FLOW REDUCTION [Qout/Qin](%)=	3.90
TIME SHIFT OF PEAK FLOW (min)=	145.00
MAXIMUM STORAGE USED (ha.m.)=	0.2860

** SIMULATION:Run 14 - 5yr 24hr 15min SCS Type II (MTO) **

RESERVOIR(0501)	OVERFLOW IS OFF
IN= 2---> OUT= 1	
DT= 5.0 min	
OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000
0.0000	0.0300
0.0100	0.0700
0.0200	0.1000
0.0200	0.1800
0.0200	0.2200
OUTFLOW (cms)	STORAGE (ha.m.)
0.3400	0.4900
0.3500	0.5400
0.3700	0.5900
0.3800	0.6400
0.4100	0.7400
0.4200	0.7900

	0.0300	0.2600		0.4400	0.8400
	0.0800	0.3500		3.9400	0.9500
	0.1800	0.4000		6.9400	1.0100
	0.3200	0.4400		10.7200	1.0600
	AREA	QPEAK		TPEAK	R.V.
	(ha)	(cms)		(hrs)	(mm)
INFLOW : ID= 2 (0201)	12.440	1.676		12.25	48.19
OUTFLOW: ID= 1 (0501)	12.440	0.130		13.42	45.70
PEAK FLOW REDUCTION [Qout/Qin](%)=	7.75				
TIME SHIFT OF PEAK FLOW (min)=	70.00				
MAXIMUM STORAGE USED (ha.m.)=	0.3750				

** SIMULATION:Run 15 - 10yr 24hr 15min SCS Type II (MTO) **

RESERVOIR(0501)	OVERFLOW IS OFF
IN= 2---> OUT= 1	
DT= 5.0 min	
OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000
0.0000	0.0300
0.0100	0.0700
0.0200	0.1000
0.0200	0.1800
0.0200	0.2200
0.0300	0.2600
0.0800	0.3500
0.1800	0.4000
0.3200	0.4400
OUTFLOW (cms)	STORAGE (ha.m.)
0.3400	0.4900
0.3500	0.5400
0.3700	0.5900
0.3800	0.6400
0.4100	0.7400
0.4200	0.7900
0.4400	0.8400
3.9400	0.9500
6.9400	1.0100
10.7200	1.0600
AREA (ha)	QPEAK (cms)
12.440	2.048
12.440	0.259
TPEAK (hrs)	R.V. (mm)
12.25	58.22
13.00	55.73
PEAK FLOW REDUCTION [Qout/Qin](%)=	12.65
TIME SHIFT OF PEAK FLOW (min)=	45.00
MAXIMUM STORAGE USED (ha.m.)=	0.4228

** SIMULATION:Run 16 - 25yr 24hr 15min SCS Type II (MTO) **

RESERVOIR(0501)	OVERFLOW IS OFF
IN= 2---> OUT= 1	
DT= 5.0 min	
OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000
0.0000	0.0300
0.0100	0.0700
0.0200	0.1000
0.0200	0.1800
0.0200	0.2200
0.0300	0.2600
0.0800	0.3500
0.1800	0.4000
0.3200	0.4400
OUTFLOW (cms)	STORAGE (ha.m.)
0.3400	0.4900
0.3500	0.5400
0.3700	0.5900
0.3800	0.6400
0.4100	0.7400
0.4200	0.7900
0.4400	0.8400
3.9400	0.9500
6.9400	1.0100
10.7200	1.0600
AREA (ha)	QPEAK (cms)
12.440	2.546
12.440	0.342
TPEAK (hrs)	R.V. (mm)
12.25	71.42
12.92	68.93
PEAK FLOW REDUCTION [Qout/Qin](%)=	13.41
TIME SHIFT OF PEAK FLOW (min)=	40.00
MAXIMUM STORAGE USED (ha.m.)=	0.4977

** SIMULATION:Run 17 - 50yr 24hr 15min SCS Type II (MTO) **

RESERVOIR(0501)	OVERFLOW IS OFF
IN= 2---> OUT= 1	
DT= 5.0 min	
OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000
0.0000	0.0300
0.0100	0.0700

	0.0200	0.1000		0.3800	0.6400
	0.0200	0.1800		0.4100	0.7400
	0.0200	0.2200		0.4200	0.7900
	0.0300	0.2600		0.4400	0.8400
	0.0800	0.3500		3.9400	0.9500
	0.1800	0.4000		6.9400	1.0100
	0.3200	0.4400		10.7200	1.0600
	AREA	QPEAK		TPEAK	R.V.
	(ha)	(cms)		(hrs)	(mm)
INFLOW : ID= 2 (0201)	12.440	3.168		12.25	81.34
OUTFLOW: ID= 1 (0501)	12.440	0.362		12.92	78.84
PEAK FLOW REDUCTION [Qout/Qin](%)=	11.43				
TIME SHIFT OF PEAK FLOW (min)=	40.00				
MAXIMUM STORAGE USED (ha.m.)=	0.5704				

** SIMULATION:Run 18 - 100yr 24hr 15min SCS Type II (MTO) **					

RESERVOIR(0501)	OVERFLOW IS OFF				
IN= 2--> OUT= 1					
DT= 5.0 min					
	OUTFLOW	STORAGE		OUTFLOW	STORAGE
	(cms)	(ha.m.)		(cms)	(ha.m.)
	0.0000	0.0000		0.3400	0.4900
	0.0000	0.0300		0.3500	0.5400
	0.0100	0.0700		0.3700	0.5900
	0.0200	0.1000		0.3800	0.6400
	0.0200	0.1800		0.4100	0.7400
	0.0200	0.2200		0.4200	0.7900
	0.0300	0.2600		0.4400	0.8400
	0.0800	0.3500		3.9400	0.9500
	0.1800	0.4000		6.9400	1.0100
	0.3200	0.4400		10.7200	1.0600
	AREA	QPEAK		TPEAK	R.V.
	(ha)	(cms)		(hrs)	(mm)
INFLOW : ID= 2 (0201)	12.440	3.595		12.25	91.43
OUTFLOW: ID= 1 (0501)	12.440	0.381		12.92	88.93
PEAK FLOW REDUCTION [Qout/Qin](%)=	10.60				
TIME SHIFT OF PEAK FLOW (min)=	40.00				
MAXIMUM STORAGE USED (ha.m.)=	0.6442				

** SIMULATION:Haze1
**

RESERVOIR(0501)				
IN= 2---> OUT= 1				
DT= 5.0 min				

OVERFLOW IS OFF				
OUTFLOW		STORAGE	OUTFLOW	STORAGE
(cms)		(ha.m.)	(cms)	(ha.m.)
0.0000		0.0000	0.3400	0.4900
0.0000		0.0300	0.3500	0.5400
0.0100		0.0700	0.3700	0.5900
0.0200		0.1000	0.3800	0.6400
0.0200		0.1800	0.4100	0.7400
0.0200		0.2200	0.4200	0.7900
0.0300		0.2600	0.4400	0.8400
0.0800		0.3500	3.9400	0.9500
0.1800		0.4000	6.9400	1.0100
0.3200		0.4400	10.7200	1.0600
		AREA	QPEAK	TPEAK
		(ha)	(cms)	(hrs)
INFLOW : ID= 2 (0201)		12.440	1.765	10.00
OUTFLOW: ID= 1 (0501)		12.440	1.718	10.00
PEAK FLOW REDUCTION [Qout/Qin](%)= 97.33				
TIME SHIFT OF PEAK FLOW (min)= 0.00				
MAXIMUM STORAGE USED (ha.m.)= 0.8809				

Appendix E: Water Quality Calculations

Water Quality Requirements

Project Details

Maple Bridge Subdivision, Phase 2	422492
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Prepared By

LJC	10/10/2024
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Water Quality Sizing Criteria

Methodology & Data Source	Volumetric water quality criteria as presented in Table 3.2 in Ministry of Environment, Conservation and Parks (MECP) Stormwater Management Planning & Design Manual (SWMPDM) March 2003.
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Contributing Catchments

Catchment ID	Area (ha)	Impervious (%)
201	12.44	71%
Total	12.44	71%

Treatment Method Details

SWM Facility Type	Wet Pond
Target Treatment Level	Enhanced Level
Treatment Percentage	80%

Treatment

Water Quality Storage Requirement	2,194 m ³
Extended Detention Volume (40 m³)	498 m ³
Permanent Pool Volume Required	1,696 m ³
25 mm Storm Runoff Depth	9 mm
25 mm Storm Runoff Volume	1,128 m ³
Required Extended Detention Volume	1,128 m ³
Erosion Control Storage Required	2,239 m ³

Permanent Pool Volume Provided 3,940 **Provided > Required**

Extended Detention Storage Provided 3,068 **Provided > Required**

Active Storage Provided 10,646 **Provided > Required**

PROJECT	Maple Bridge Subdivision, Phase 2	FILE	422492		
		DATE	10/10/2024		
SUBJECT	Water Quality - Treatment Train Calculations	NAME	LJC		
		PAGE	1	OF	1

Water Quality Treatment Train Calculations

Catchment 201

Total Drainage Area Conveyed to Controls: 12.44 ha

Total Imperviousness Conveyed to Controls: 71%

Device		Target Total Suspended Solids (TSS) Removal
Primary Treatment	Wet Pond	80%

Note: TSS removal efficiencies obtained from the Sustainable Technologies Evaluation Program (STEP) Low Impact Development Treatment Tool.

TSS Removal Provided By Controls: 80%

Catchment 202

Due to grading constraints, Catchment 202 drains uncontrolled to Outlet #1. In addition, the catchment consists of clean landscape area. Therefore, controls are not required.

Catchment 203

Due to grading constraints, Catchment 203 drains uncontrolled to Outlet #1. Best efforts were made to minimize the post-development flows discharging to the outlet and therefore minimize the discharge of TSS from this catchment.

Appendix F: Soakaway Pit Design

PROJECT	Maple Bridge Subdivision, Phase 2	FILE	422492
		DATE	4/4/2024
SUBJECT	Soakaway Pit Sizing	NAME	LJC/JG
		PAGE	1 OF 1

SINGLE FAMILY DWELLINGS (36' AND 46' CLASSIC)

Soakaway Pit Length	3.2 m
Soakaway Pit Width	2.0 m
Pipe Diameter	0.15 m
Clearstone Height	1.2 m
Clearstone Void Ratio	0.4

Pipe Storage Volume	0.06 m ³
Clearstone Storage Volume	3.05 m ³
Total Storage Volume	3.11 m ³

Total Site Impervious Area	8.20 ha
Number of Soakaway Pits	38

Total Storage Volume	118 m³
Equivalent Storm Captured	1.4 mm

TOWNHOUSES

Soakaway Pit Length	1.3 m
Soakaway Pit Width	2.0 m
Pipe Diameter	0.15 m
Clearstone Height	1.2 m
Clearstone Void Ratio	0.4

Pipe Storage Volume	0.02 m ³
Clearstone Storage Volume	1.24 m ³
Total Storage Volume	1.26 m ³

Total Site Impervious Area	8.20 ha
Number of Soakaway Pits	61

Total Storage Volume	77 m³
Equivalent Storm Captured	0.9 mm

Total Storage Volume	195 m³
Total Equivalent Storm Captured	2.4 mm

PROJECT	Maple Bridge Subdivision, Phase 2	FILE	422492
		DATE	3/6/2025
SUBJECT	Drawdown Time - Infiltration Galleries	NAME	LJC
		PAGE	1 OF 1

Calculations completed in accordance with:

Infiltration: Sizing and modeling: LID SWM Planning and Design Guide. (2021, December 10). Sustainable Technologies Evaluation Program. Retrieved October 11, 2024 from:
[https://wiki.sustainabletechnologies.ca/index.php?title=Infiltration: Sizing and modeling&oldid=12158](https://wiki.sustainabletechnologies.ca/index.php?title=Infiltration:_Sizing_and_modeling&oldid=12158)

Infiltration Sizing when practice is fixed or constrained (1D Drainage)

Ratio of Catchment Impervious Area to Practice Permeable Area

$$R = \frac{A_i}{A_p}$$

Where:

A_i = Catchment Impervious Area = 0.77 ha
 A_p = Area of Practice (m²) = 402 m²

R = 19.2

Drawdown Time Within Practice Permeable Area

$$d_r = \frac{f' \times t}{n}$$

Where:

d_r : reservoir depth (m)
 f' : Design Infiltration Rate (per Section 4.2.3 of GHD's Hydrogeological Assessment Addendum, 2025)
 n : Porosity of Storage Material

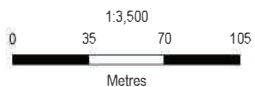
Composite Porosity

Infiltration Chamber d_r : 1.20 m (clearstone media)
 n : 40%

Drawdown Time

d_r : 1.20 m
 f' : 16 mm/hr
 n : 40.0%
 t = 30 hours

Total Drawdown Time: 30 hours



Map Projection: Transverse Mercator
Horizontal Datum: North American 1983
Grid: NAD 1983 UTM Zone 17N

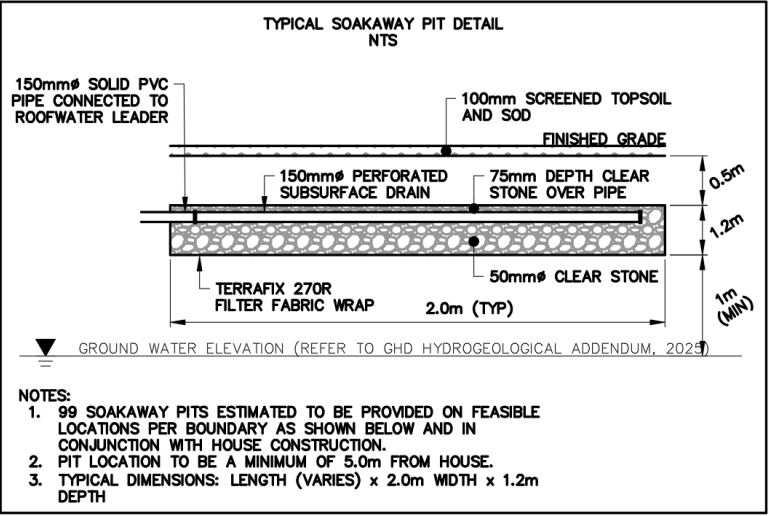
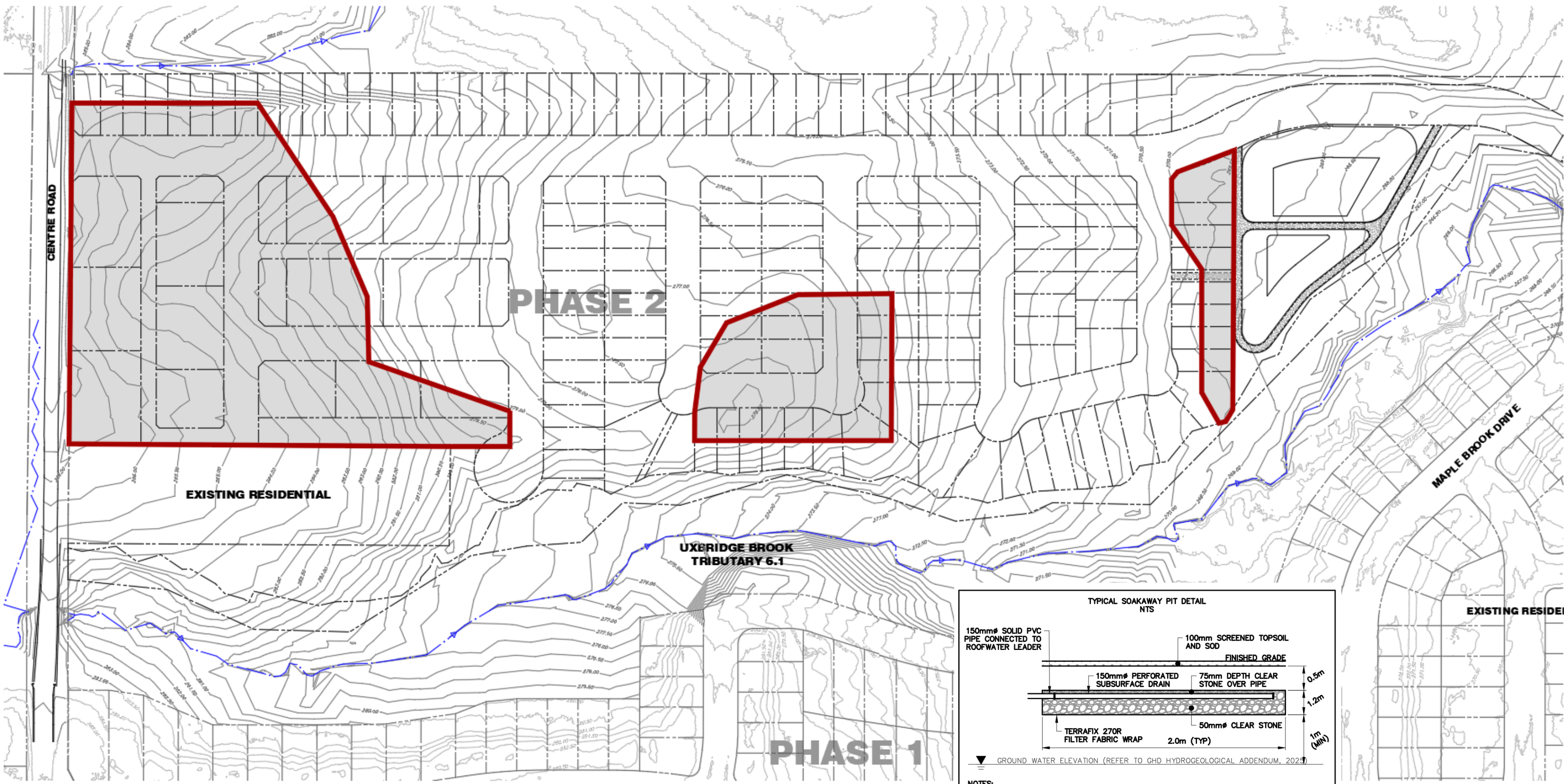


Mason Homes (Uxbridge) Limited
7309 Centre Road, Uxbridge, Ontario
Municipality of Durham

Hydrogeological Assessment Update
Groundwater Elevations

Project No. 12641133
Revision No.
Date Jan 31, 2025

Figure 5



NOTE: DRAWING NOT TO SCALE

MAPLE BRIDGE PHASE 2 - PRELIMINARY STORMWATER MANAGEMENT REPORT
CONCEPTUAL SOAKAWAY PIT LOCATIONS

LOCATIONS AND DETAILS AS PER THE HYDROGEOLOGICAL ASSESSMENT ADDENDUM PREPARED BY GHD (2025). DETAILS WILL BE CONFIRMED AT DETAILED DESIGN.



Appendix G: Phosphorus Balance

Loading Summary TP | Maple Bridge Subdivision, Phase 2 - Existing Conditions

Catchment 1	-0.656 %	0.014 m ³ /s	5,857.14 m ³	457 m ³
			0.224 mg/l	0.225 mg/l
			1.312 kg	0.103 kg
Catchment 2	0.043 %	0.052 m ³ /s	122,244.18 m ³	9,364 m ³
			0.229 mg/l	0.229 mg/l
			27.984 kg	2.143 kg
Total	92.334 %	0.066 m ³ /s	128,101.32 m ³	9,821 m ³
			0.229 mg/l	0.229 mg/l
			29.296 kg	2.246 kg

Loading TP | Maple Bridge Subdivision, Phase 2 - Existing Conditions

TP - Catchment 1

Catchment 100	0 %	0.01 m ³ /s	5,857.14 m ³	460 m ³
			0.224 mg/l	0.224 mg/l
			1.312 kg	0.103 kg
Outlet #2	0 %	0.014 m ³ /s	457 m ³	457 m ³
			0.225 mg/l	0.225 mg/l
			0.103 kg	0.103 kg

Name	LID Type	Peak Outflow	Incoming	Outgoing
			Total Flow (m ³)	Total Flow (m ³)
			Concentration (mg/l)	Concentration (mg/l)
			Total Load (kg)	Total Load (kg)
Catchment 101	0 %	0.05 m ³ /s	122,244.18 m ³	9,360 m ³
			0.229 mg/l	0.229 mg/l
			27,984 kg	2,143 kg
Outlet #1	0 %	0.052 m ³ /s	9,364 m ³	9,364 m ³
			0.229 mg/l	0.229 mg/l
			2,143 kg	2,143 kg

Loading Summary TP | Maple Bridge Subdivision, Phase 2- Proposed Conditions

Catchment	Total Catchment TP Removal	Peak Outflow	Generated	Outgoing
			Total Flow (m ³)	Total Flow (m ³)
			Average Concentration (mg/l)	Average Concentration (mg/l)
			Total Load (kg)	Total Load (kg)
Catchment 1	0.919 %	1.125 m ³ /s	128,101.32 m ³	63,119 m ³
			0.252 mg/l	0.249 mg/l
			32.241 kg	15.74 kg
Total	51.18 %	1.125 m ³ /s	128,101.32 m ³	63,119 m ³
			0.252 mg/l	0.249 mg/l
			32.241 kg	15.74 kg

Loading TP | Maple Bridge Subdivision, Phase 2- Proposed Conditions

TP - Catchment 1

Name	LID Type	Peak Outflow	Incoming	Outgoing
			Total Flow (m ³)	Total Flow (m ³)
			Concentration (mg/l)	Concentration (mg/l)
			Total Load (kg)	Total Load (kg)
Catchment 203	0 %	0.04 m ³ /s	4,062.21 m ³	1,780 m ³
			0.278 mg/l	0.278 mg/l
			1.13 kg	0.495 kg
Catchment 202	0 %	0.02 m ³ /s	15,115.2 m ³	1,220 m ³
			0.271 mg/l	0.271 mg/l
			4.089 kg	0.33 kg
Catchment 201	0 %	1.07 m ³ /s	108,923.91 m ³	60,120 m ³
			0.248 mg/l	0.248 mg/l
			27.022 kg	14.915 kg
Outlet #1	0 %	1.125 m ³ /s	63,119 m ³	63,119 m ³
			0.249 mg/l 15.74 kg	0.249 mg/l 15.74 kg

Loading Summary TP | Maple Bridge Subdivision, Phase 2 - Mitigation Measures

Catchment	Total Catchment TP Removal	Peak Outflow	Generated	Outgoing
			Total Flow (m ³)	Total Flow (m ³)
			Average Concentration (mg/l)	Average Concentration (mg/l)
			Total Load (kg)	Total Load (kg)
Catchment 1	55.049 %	0.157 m ³ /s	128,101.32 m ³	49,391 m ³
			0.252 mg/l	0.113 mg/l
			32.247 kg	5.589 kg
Total	82.668 %	0.157 m ³ /s	128,101.32 m ³	49,391 m ³
			0.252 mg/l	0.113 mg/l
			32.247 kg	5.589 kg

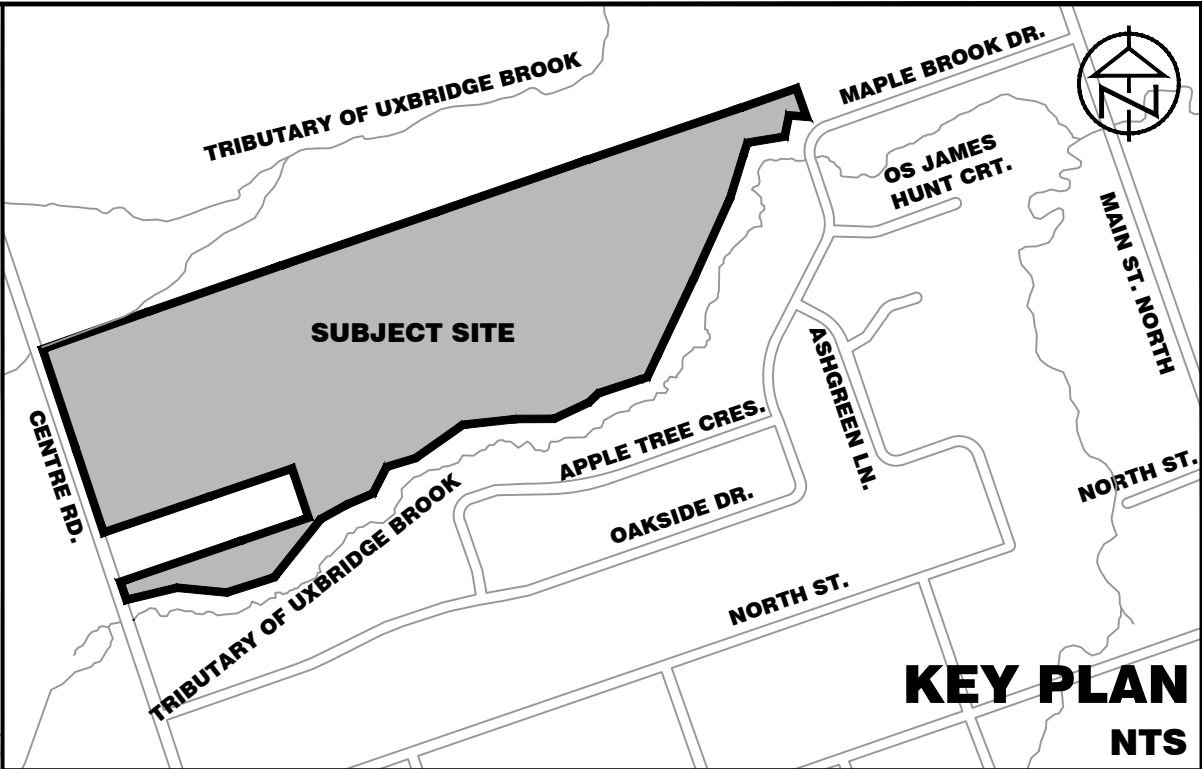
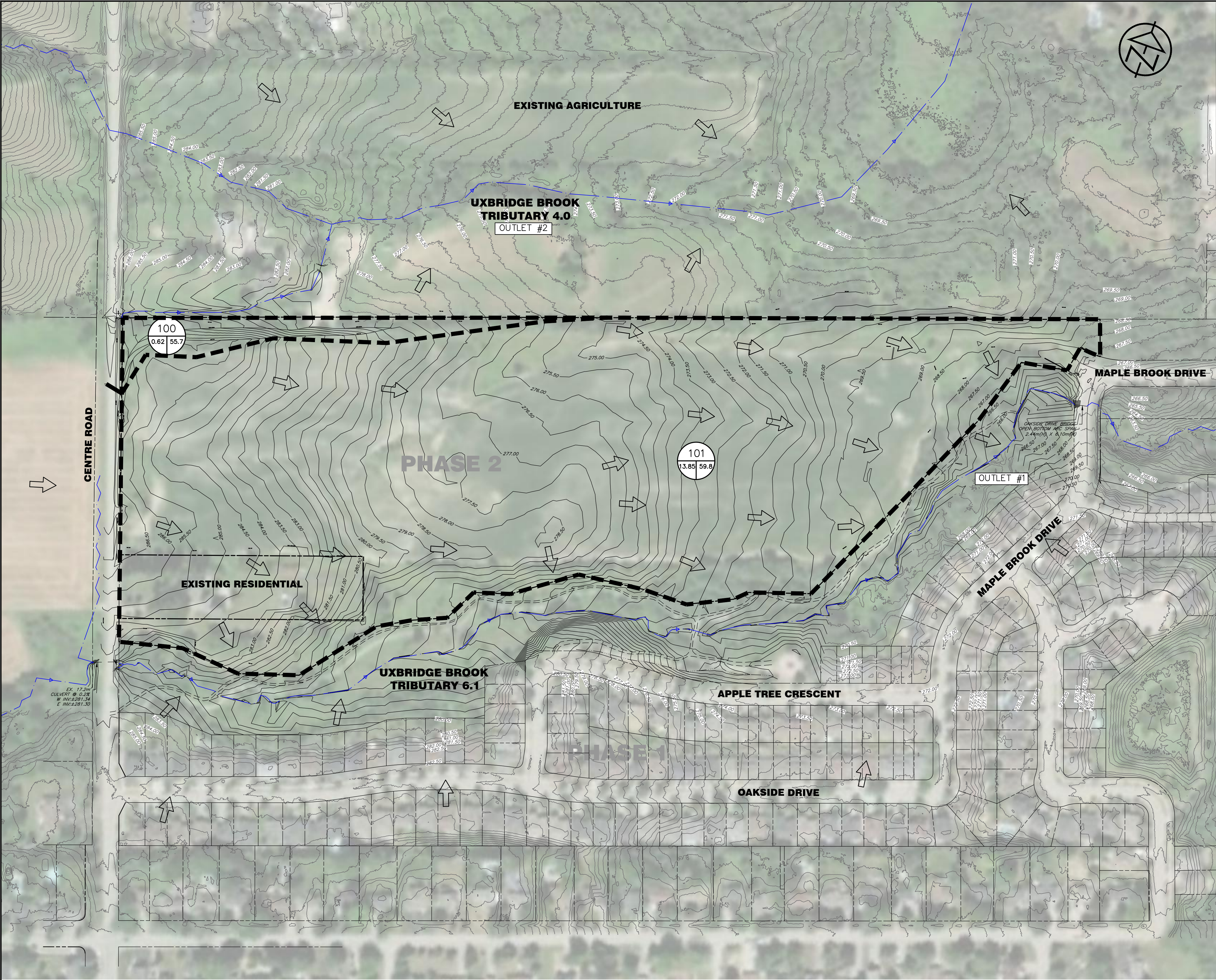
Loading TP | Maple Bridge Subdivision, Phase 2 - Mitigation Measures

TP - Catchment 1

Name	LID Type	Peak Outflow	Incoming	Outgoing
			Total Flow (m ³)	Total Flow (m ³)
			Concentration (mg/l)	Concentration (mg/l)
			Total Load (kg)	Total Load (kg)
Catchment 203	0 %	0.04 m ³ /s	4,062.21 m ³	1,780 m ³
			0.278 mg/l	0.278 mg/l
			1.13 kg	0.495 kg
Catchment 202	0 %	0.02 m ³ /s	15,115.2 m ³	1,220 m ³
			0.271 mg/l	0.271 mg/l
			4.089 kg	0.33 kg
Catchment 201	0 %	0.95 m ³ /s	100,893.96 m ³	53,880 m ³
			0.261 mg/l	0.261 mg/l
			26.306 kg	14.048 kg

Soakaways	87 %	0 m ³ /s	8.029,95 m ³	0 m ³
			0.09 mg/l	0.012 mg/l
			0.723 kg	0 kg
Proposed Wet Pond	63 %	0.955 m ³ /s	53.900 m ³	53.900 m ³
			0.261 mg/l	0.096 mg/l
			14.048 kg	5.198 kg
Wet Pond to Outlet	0 %	0.097 m ³ /s	49.400 m ³	49.400 m ³
			0.096 mg/l	0.096 mg/l
			4.764 kg	4.764 kg
Outlet #1	0 %	0.157 m ³ /s	49.391 m ³	49.391 m ³
			0.113 mg/l	0.113 mg/l
			5.589 kg	5.589 kg

Drawings



- LEGEND**
- 285.00 — EXISTING CONTOURS (0.5m INTERVALS)
 - 285.00 — LSRCA CONTOURS (5.0m INTERVALS)
 - — EXISTING WATERCOURSE AND FLOW DIRECTION
 - — DRAINAGE CATCHMENT BOUNDARY
 - EXISTING OVERLAND FLOW DIRECTION
 - 101 — DRAINAGE CATCHMENT ID
 - 13.85 59.8 — CURVE NUMBER
 - DRAINAGE AREA (ha)

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DRAWING REFERENCES

TOPOGRAPHIC FEATURES, CONTOURS AND LEGAL BOUNDARIES SHOWN ON THIS PLAN BASED ON SURVEY COMPLETED BY IBW SURVEYORS LTD., COMPLETED ON SEPTEMBER 10, 2019 AND H.F.GRANDER Co. LTD., COMPLETED AND APRIL 6, 2023.

SUPPLEMENTARY SURVEY DATA GENERATED FROM LSRCA DIGITAL TERRAIN MODEL. REFER TO NATURAL HAZARD ASSESSMENT PREPARED BY TATHAM ENGINEERING LIMITED FOR DETAILS.

DRAFT PLAN INFORMATION SHOWN ON THIS PLAN BASED ON PLAN PROVIDED BY IPS, DATED MARCH 11, 2024.

No.	REVISION DESCRIPTION	DATE
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2.	RE-ISSUED FOR DRAFT PLAN APPROVAL	AUG. 19/25

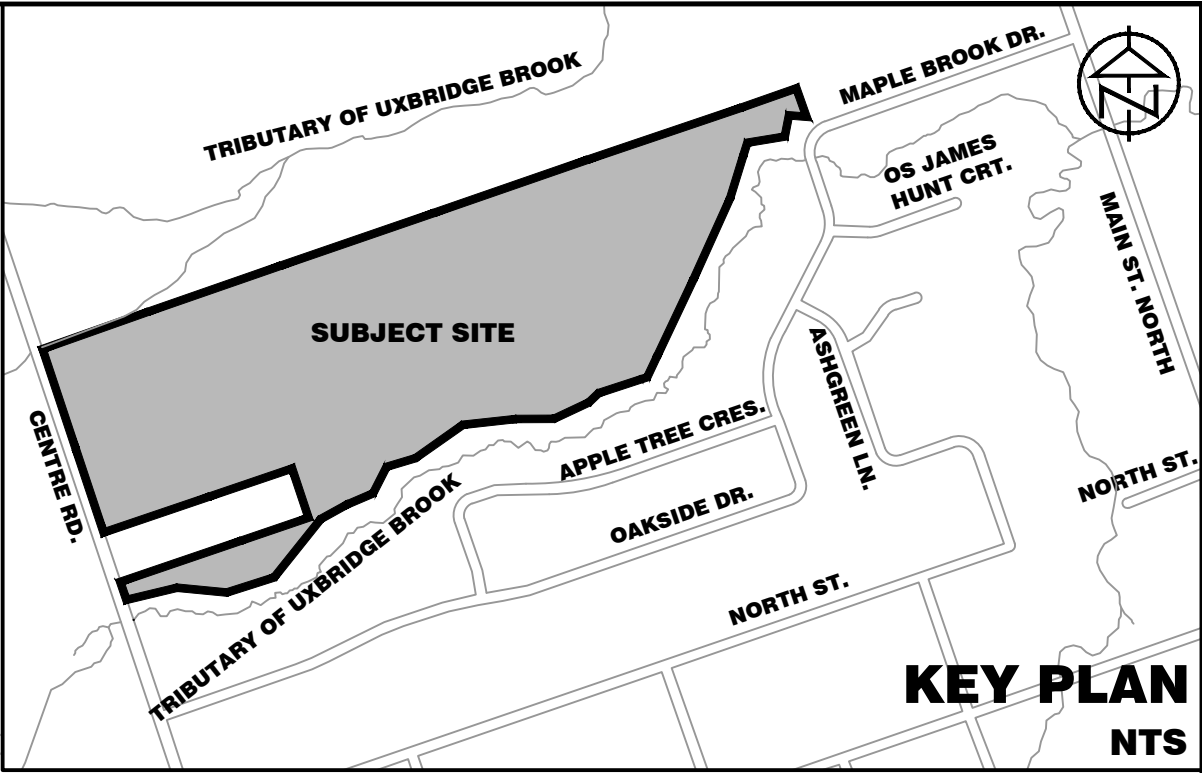
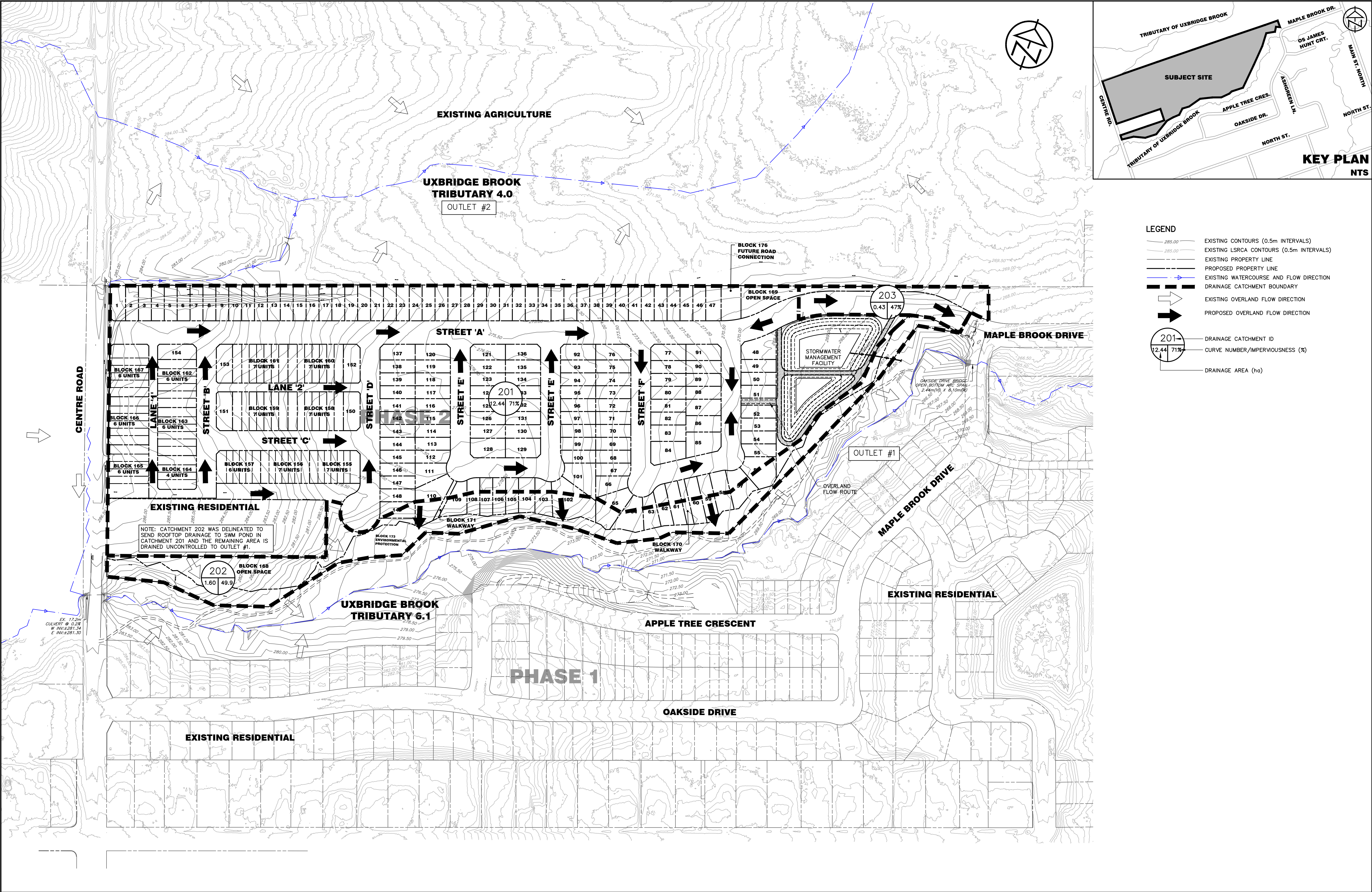
ENGINEER STAMP

MAPLE BRIDGE RESIDENTIAL DEVELOPMENT - PHASE 2 TOWNSHIP OF UXBRIDGE REGIONAL MUNICIPALITY OF DURHAM MASON HOMES LIMITED
PRE-DEVELOPMENT DRAINAGE PLAN



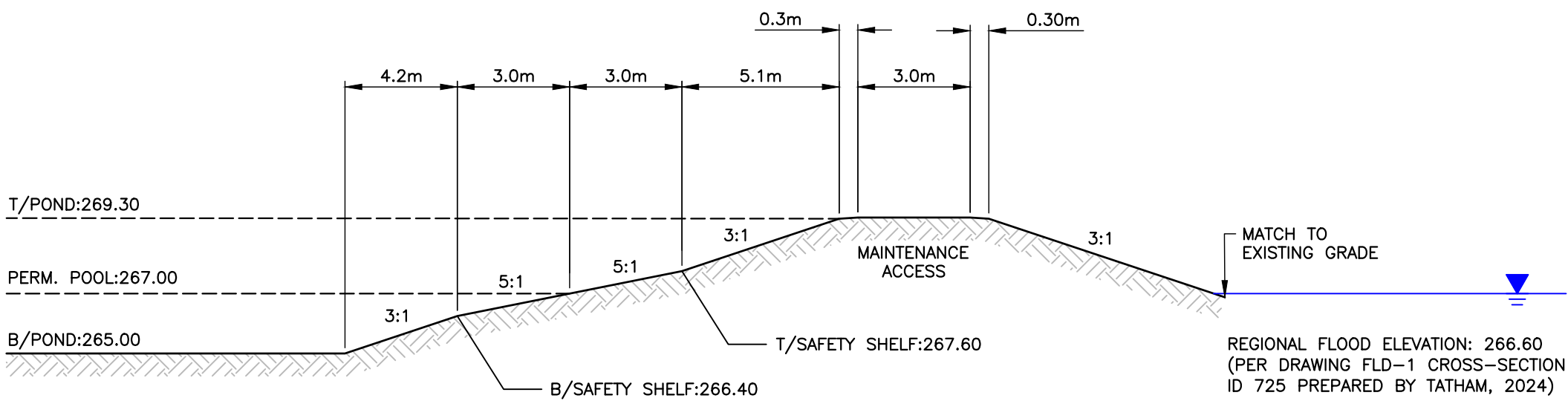
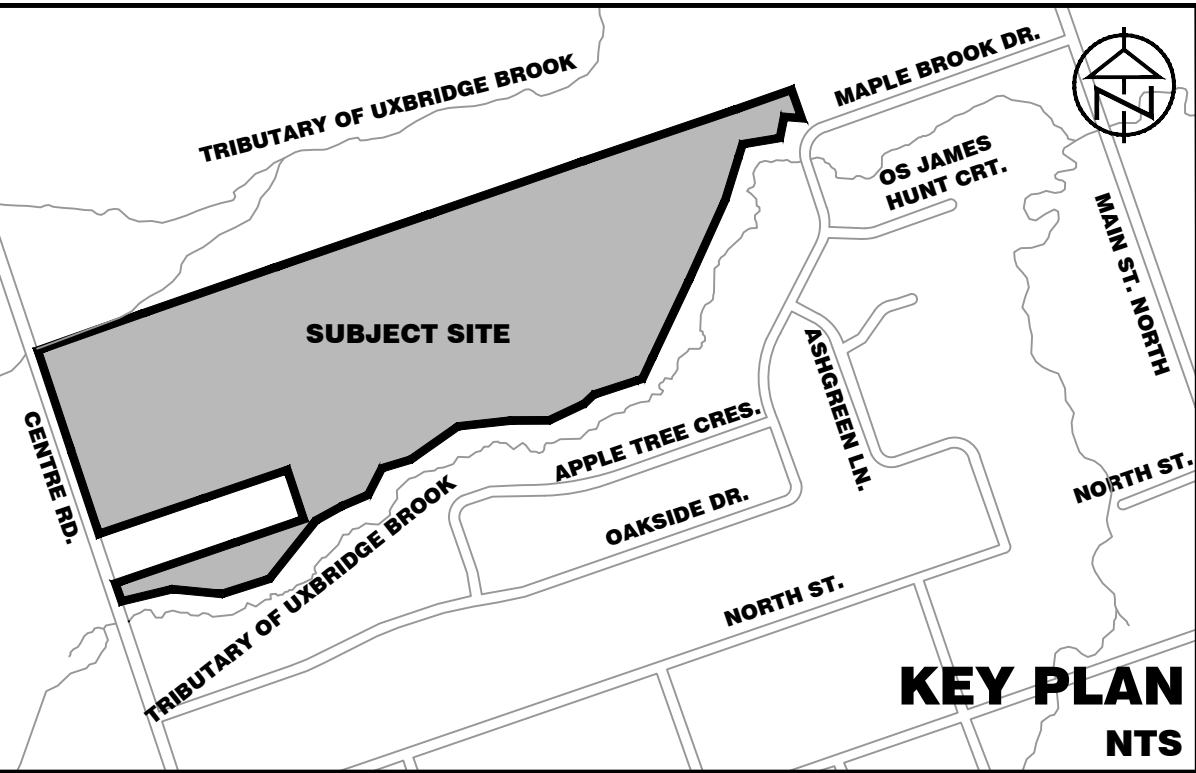
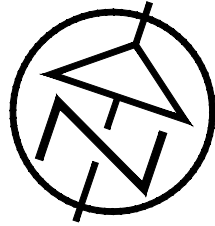
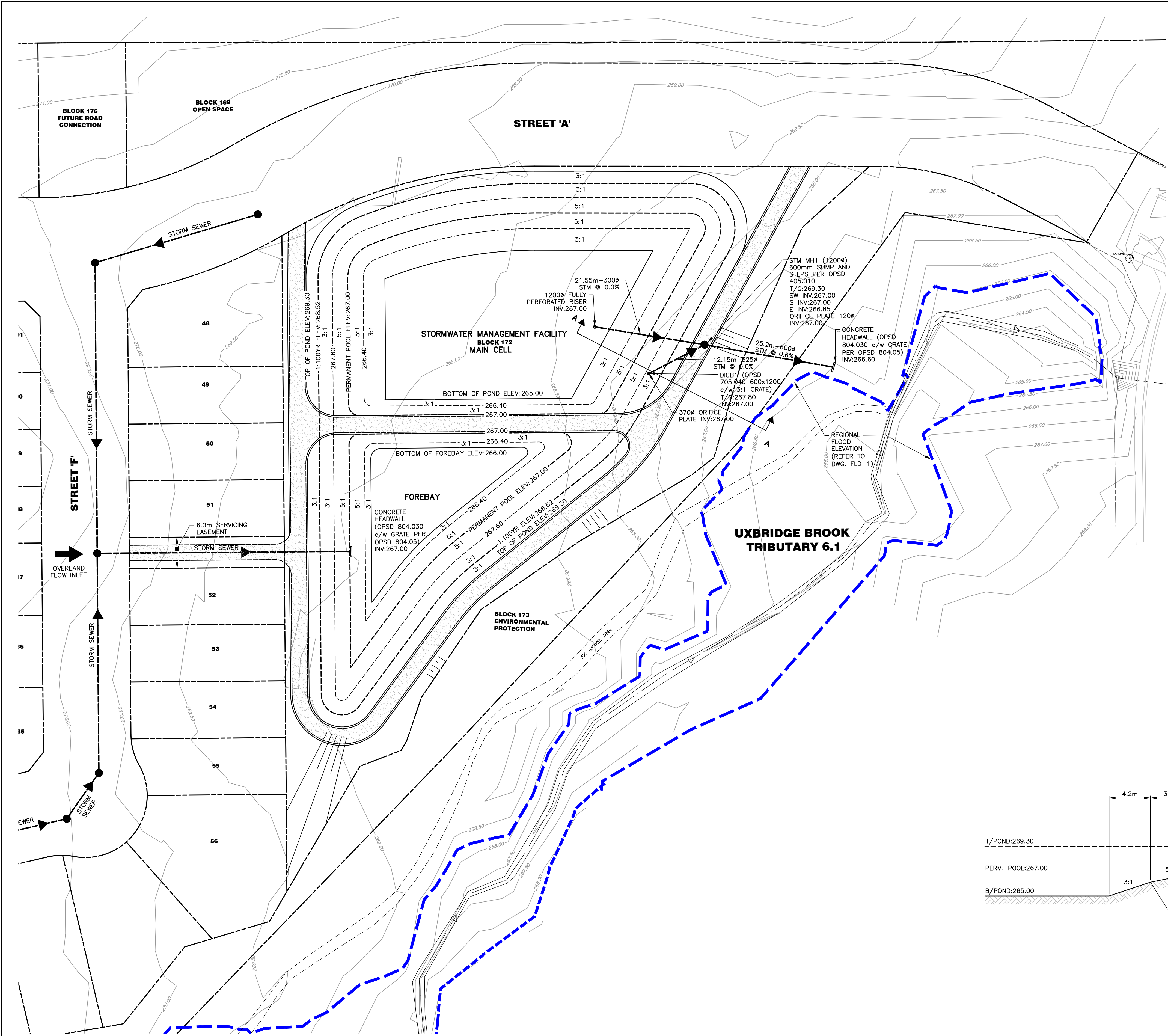
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DP-1

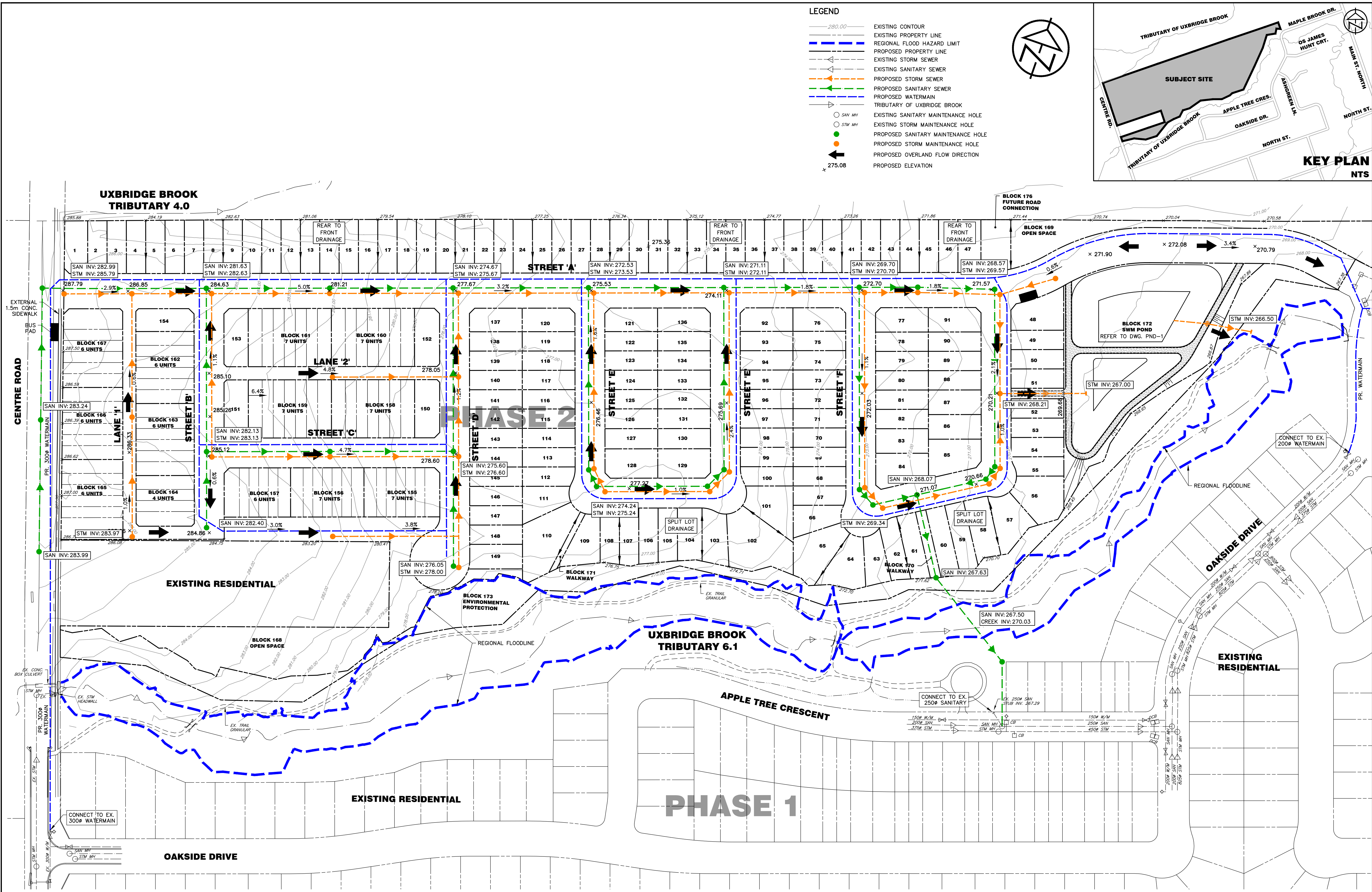


- LEGEND**
- 285.00 ——— EXISTING CONTOURS (0.5m INTERVALS)
 - 285.00 ——— EXISTING LSRCA CONTOURS (0.5m INTERVALS)
 - — — — — EXISTING PROPERTY LINE
 - — — — — PROPOSED PROPERTY LINE
 - — — — — EXISTING WATERCOURSE AND FLOW DIRECTION
 - — — — — DRAINAGE CATCHMENT BOUNDARY
 - EXISTING OVERLAND FLOW DIRECTION
 - PROPOSED OVERLAND FLOW DIRECTION
 - 201 ——— DRAINAGE CATCHMENT ID
 - 12.44 71% ——— CURVE NUMBER/IMPERVIOUSNESS (%)
 - DRAINAGE AREA (ha)

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		2.	RE-ISSUED FOR DRAFT PLAN APPROVAL	AUG. 19/25						



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		1.	ISSUED FOR DRAFT PLAN APPROVAL	APR. 17/24			
		2.	RE-ISSUED FOR DRAFT PLAN APPROVAL	AUG. 19/25			
STORMWATER MANAGEMENT FACILITY							



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		1.	ISSUED FOR DRAFT PLAN APPROVAL	APR. 17/24					
		2.	RE-ISSUED FOR DRAFT PLAN APPROVAL	AUG. 19/25					
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